

GENERAL STRUCTURAL NOTES

- SLAB ELEVATIONS SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS.
- BUILDING DIMENSIONS SHALL BE AS PER ARCHITECTURAL DRAWINGS UNLESS SHOWN OTHERWISE
- TYPICAL DETAILS SHOWN ON SDC SHEETS SHALL APPLY TO ALL DRAWINGS UNLESS NOTED OTHERWISE.
- UNLESS NOTED OTHERWISE, REFER TO DRAWINGS OTHER THAN STRUCTURAL FOR FINISHES, SLOPES, DEPRESSIONS, OPENING CURBS, STAIRS, RAMPS, TRENCHES, EQUIPMENT AND LOCATIONS AND EXTENT OF SUCH CONDITIONS.
- CONTRACTOR TO COORDINATE ALL NEW WORK WITH EXISTING SITE CONDITIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- DETAILS OR CONDITIONS NOT FULLY DEVELOPED ON STRUCTURAL DOCUMENTS ARE SIMILAR TO DEVELOPED DETAILS.
- SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR WATERPROOFING, DAMP-PROOFING, AND DRAINAGE REQUIREMENTS.
- REFER TO GEOTECHNICAL REPORT FOR SITE CONDITIONS EXCAVATION, SHORING REQUIREMENTS, UNDERPINNING, BACKFILL BEHIND WALLS AND SUBDRAINAGE PREPARATIONS.
- ALL BUILDING FOUNDATION PLANS AND ROOF PLANS TO BE COORDINATED WITH GENERAL NOTES AND TYPICAL DETAILS AS APPLICABLE.
- STRUCTURES HAVE BEEN DESIGNED TO BE STABLE AND SELF SUPPORTING AFTER THE CONSTRUCTION IS COMPLETE, IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY FOR THE BUILDING'S STABILITY DURING CONSTRUCTION. THIS RESPONSIBILITY ALSO INCLUDES BUT IS NOT LIMITED TO METHOD AND SEQUENCE OF ERECTION, TEMPORARY SHORING AND TEMPORARY BRACING.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- THE REFERENCE DATUM FOR ALL ELEVATIONS IN THIS STRUCTURAL PLAN SET IS BASED FROM THE DATUM USED IN THE CIVIL PLAN SET.

SPECIAL INSPECTION

- SPECIAL INSPECTION IN ACCORDANCE WITH 2009 IBC CHAPTER 17 IS REQUIRED ON THE FOLLOWING PORTIONS OF THE WORK:
 CONCRETE REINFORCING STEEL
 SOILS STRUCTURAL STEEL
 REFER TO THE STATEMENT OF SPECIAL INSPECTIONS FOR MORE SPECIFIC REQUIREMENTS.

CONCRETE

- CAST IN PLACE CONCRETE:
 FOUNDATION $f_c = 4,000$ psi
 SLAB ON GRADE $f_c = 4,000$ psi
 WALLS, COLUMNS, BEAMS, SLABS $f_c = 4,000$ psi
 CONCRETE TOPPING OVER METAL DECK $f_c = 4,000$ psi
 OTHERS $f_c = 3,000$ psi
 UNLESS NOTED OTHERWISE ON PLANS
- CONCRETE REINFORCING CLEAR COVER SHALL BE AS FOLLOWS:
 CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3 INCHES
 CONCRETE EXPOSED TO EARTH OR WEATHER:
 NO. 6 OR LARGER BARS 2 INCHES
 NO. 6 OR SMALLER BARS 1.5 INCHES
 CONCRETE NOT EXPOSED TO EARTH OR IN CONTACT WITH GROUND:
 SLAB OR WALLS 0.75 INCHES
 BEAMS OR COLUMN PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1.5 INCHES
- ALL CONCRETE DIMENSIONS SHOWN ARE MINIMUM DIMENSIONS. CONTRACTOR TO REVIEW FORMING, REINFORCING, REINFORCING DETAILS AND ANY EMBEDDED ITEMS AND DETERMINE PRIOR TO FABRICATION OF ANY REINFORCING, PLACEMENT REQUIREMENTS AND CLEARANCES.
- CONCRETE MASONRY UNIT (CMU):
 COMPOSITE CMU (MIN.) $f_m = 1,500$ psi

CONCRETE AND MASONRY REINFORCEMENT

- ALL CONCRETE REINFORCING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706. ASTM A615 STEEL MAY BE USED IF IT CONFORMS TO ALL REQUIREMENTS OF ACI 318-08 SECTION 21.1.5.
- REINFORCING SHALL EXTEND CONTINUOUS FOR THE DIMENSION SHOWN.
- NO WELDING OF ANY REINFORCING IS PERMITTED, UNLESS SPECIFICALLY STATED ON THE PLANS. REINFORCEMENT TO BE WELDED TO MEET THE REQUIREMENTS OF ASTM A706.
- LOCATE ALL REINFORCING AS SHOWN ON DRAWINGS AND FASTEN SECURELY.
- ALL REINFORCING TO TERMINATE WITH STANDARD HOOKS AS SHOWN ON PLANS. ALL STIRRUPS AND TIES TO BE CLOSED WITH 135 DEGREE BENDS.

POST INSTALLED ANCHORS

- SEE SPECIFICATIONS SECTION 03 15 00.

FOUNDATION

- UNLESS DIFFERENT VALUES ARE PROVIDED IN THE PROJECT GEOTECHNICAL REPORT, THE FOLLOWING SOIL PROPERTIES SHALL BE ASSUMED:
 DL + LL 4,000 psf
 DL + LL + WIND OR EARTHQUAKE 6,000 psf
 COEFFICIENT OF FRICTION 0.35
 PASSIVE SOIL RESISTANCE 250 psf/ft
 (SEE EARTHWORK SPECIFICATIONS FOR EARTHWORK, FILL MATERIALS, COMPACTION REQUIREMENTS AND BASECOURSE.)
- THE CONTRACTOR SHALL RETAIN A SOIL ENGINEER TO PREPARE A GEOTECHNICAL REPORT TO VERIFY ASSUMED VALUES HEREIN PRIOR TO COMMENCING ANY WORK.
- ALL GRADING SHALL BE DONE IN ACCORDANCE WITH THE CONTOURS AND DIMENSIONS INDICATED. SUBGRADE SHALL BE SLIGHTLY SLOPED TO PROVIDE PROPER SURFACE DRAINAGE AND TO AVOID SURFACE PONDING.

- EXCAVATION FOR NEW FOUNDATION ELEMENTS AND CONCRETE SLABS ON GROUND SHALL EXTEND DOWN TO A MINIMUM OF 24 INCHES BELOW THE BOTTOM OF THE FOUNDATION ELEMENTS SUCH THAT THE UNDERLYING WHITISH LIMESTONE IS EXPOSED, WITH ALL REDDISH BROWN SILTY SOIL BEING REMOVED. FOUNDATION ELEMENTS SHALL NOT BEAR ON BOULDERS. THE EXPOSED TOP SURFACE OF THE LIMESTONE SHALL BE SCARIFIED TO APPROXIMATELY 12 INCHES DEEP, MOISTURE CONDITIONED AND RECOMPACTED TO AT LEAST 95 PERCENT OF THE MAXIMUM DRY DENSITY. THE EXCAVATION CAN THEN BE BACKFILLED WITH APPROPRIATE MATERIAL AS DESCRIBED BELOW IN LAYERS NOT TO EXCEED 12 INCHES AND MOISTURE CONDITIONED AND THOROUGHLY COMPACTED TO A MINIMUM OF 95 PERCENT OF THE MAXIMUM DRY DENSITY.
- FOOTING EXCAVATION AND RECOMPACTION, WHERE REQUIRED, SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER TO VERIFY CONDITION OF SOIL BEARING CAPACITY PRIOR TO PLACEMENT OF FOUNDATION FORMS AND REBAR. WHERE UNSATISFACTORY SOILS ARE ENCOUNTERED, THEY SHALL BE OVEREXCAVATED AND REPLACED WITH A LEAN CONCRETE OR CEMENT GROUT OR AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
- FILL AND BACKFILL MATERIAL SHALL BE FREE OF ROCK FRAGMENTS OR LUMPS WITH MAXIMUM DIMENSION LESS THAN 3 INCHES, NON-EXPANSIVE WITH A PLASTICITY INDEX LESS THAN OR EQUAL TO 6 PERCENT AND A LIQUID LIMIT OF NOT MORE THAN 25 PERCENT, AND WITH NO MORE THAN 25 PERCENT PASSING A NO. 200 MESH SIEVE. UNSUITABLE MATERIALS SUCH AS DEBRIS AND ORGANICS SHALL NOT BE USED. ON SITE EXCAVATED SILTY SANDY LIMESTONE GRAVEL SOIL MEETING THE ABOVE REQUIREMENTS MAY BE USED AS BACKFILL MATERIAL.
- CONCRETE SLABS ON GROUND SHALL BEAR ON EITHER NATURAL LIMESTONE OR BACKFILL MATERIAL PLACED AND PREPARED AS DESCRIBED ABOVE, AND SHALL BE UNDERLAIN BY A MINIMUM OF 6 INCHES OF AGGREGATE BASE COURSE, PLACED AND COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY, WITH A UNIFORM AND NON-YIELDING SURFACE.

STEEL

- DETAIL, FABRICATE, AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LATEST EDITION AND SUPPLEMENTS).
- J ANCHOR BOLTS: ASTM F1554 GRADE 36.
- ALL STEEL BARS & PLATES SHALL BE ASTM A36 UNLESS OTHERWISE NOTED.
- ALL WIDE FLANGE SHAPES SHALL BE ASTM A992 (GRADE 50).
- ALL HSS SECTIONS SHALL BE ASTM A500; GRADE B.
- ALL PIPES TO BE ASTM A53; GRADE B.
- ALL THREADED RODS: ASTM A36 OR ASTM A572; GRADE 50.
- BOLTED CONNECTIONS, UNLESS NOTED OTHERWISE: 3/4-INCH DIAMETER A325-X BOLTS.
- INSTALL HIGH STRENGTH BOLTS IN ACCORDANCE WITH SECTION 8 OF THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS, 2009 EDITION.
- PROVIDE BEVELLED WASHERS ON ALL CONNECTION TO SLOPING FLANGES OF W SECTIONS AND CHANNELS WHERE SLOPE EXCEEDS 1:20.
- ANCHOR RODS SHALL BE THREADED ANCHOR RODS WITH NUT. THE EMBEDDED NUT SHALL BE TACK WELDED TO THE ANCHOR ROD TO PREVENT ROTATION DURING TIGHTENING.
- BOLT HOLES IN STEEL SHALL BE "STANDARD" (1/16-INCH LARGER IN DIAMETER THAN THE NOMINAL BOLT SIZE, UNLESS OTHERWISE NOTED).
- WELDING ELECTRODES (FILLER METAL) E70XX (70KSI), WITH EXACT FILLER METAL SELECTED BY THE FABRICATOR.
- WELD LENGTHS CALLED FOR ON THE PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED, WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF THE JOINT.
- COMPLETE PENETRATION WELDS SHALL BE MADE WITH PROPER BACKING WHEREVER POSSIBLE. AFTER WELDING, REMOVE BACKING BARS AND GRIND SMOOTH. FULL PENETRATION WELDS MADE WITHOUT PROPER BACKING SHALL HAVE THE ROOT GOUGED BEFORE WELDING IS STARTED FROM THE OTHER SIDE EXCEPT AS PROVIDED IN AWS D1.1.
- ALL BUTT AND GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.
- ALL SPLICING OF MEMBERS SHALL BE AS SHOWN ON THE DRAWINGS. ANY SPLICING OF STEEL MEMBERS PROPOSED BY THE STEEL FABRICATOR SHALL BE SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER PRIOR TO FABRICATION.
- ALL ANCHOR BOLTS SHALL BE EMBEDDED AS SHOWN ON THE DRAWINGS.
- MINIMUM PLATE THICKNESS IN 3/8 UNLESS OTHERWISE NOTED. MINIMUM WELD ID 1/4 INCH UNLESS OTHERWISE NOTED.
- ALL STEEL FABRICATION AND DETAILS TO COMPLY WITH MOST STRINGENT OF THE LATEST EDITION OF AISC CODE, AWS CODE, AND THE 2009 IBC.
- ALL WELDING TO BE BY AWS CERTIFIED WELDERS AND SHALL CONFORM TO ALL 2009 IBC AND AWS REQUIREMENTS. ALL WELDERS SHALL BE PRE-QUALIFIED BY THE PROJECT WELDING INSPECTOR FOR THE WELD TYPES AND POSITION USED IN THE PROCEDURES THEY WILL BE PERFORMING.
- UNLESS NOTED OTHERWISE, ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.

STRUCTURAL DESIGN CRITERIA

- REFERENCES
 IBC 2009 AISC 360-05
 ASCE 7-05 ACI 530-08
 ACI 318-08 PCI DESIGN HANDBOOK 6TH EDITION
- LOADS
 A. DEAD LOAD
 SELF WEIGHT OF THE STRUCTURE
 CONCRETE 150 pcf
 STEEL 490 pcf

STRUCTURAL DESIGN CRITERIA

- B. LIVE LOAD
 DESIGN LOADS:
 PARTITION/HVAC/PIPING FOR FLOORS & ROOF 25 psf
 ROOF LIVE LOAD 80 psf
 FLOOR/DECK/BALCONY 20 psf

- C. WIND
 ASCE 7-05
 DESIGN WIND SPEED = 170 mph
 EXPOSURE = C
 IMPORTANCE FACTOR = 1.15

- D. SEISMIC
 SEISMIC DESIGN ASCE 7-05
 $S_s = 1.5$ TRANSITION PERIOD = 12 SEC.
 $S_1 = 0.6$ SITE CLASS C
 IMPORTANCE FACTOR = 1.5
 SEISMIC DESIGN CATEGORY D

- SEISMIC FORCE RESISTING SYSTEMS:
 MAIN BUILDING - R/C SMRF
 $R = 8$ OMEGA = 3 $C_d = 5.5$
 ELEVATOR/STAIR CORE - SPECIAL R/C BEARING WALLS
 $R = 5$ OMEGA = 2.5 $C_d = 5.5$

IBC 2009 / ASCE 7-05 LOAD COMBINATIONS

LRFD (LOAD AND RESISTANCE FACTOR DESIGN)

pg 308 IBC 2009	D	H	F	L	Lr/R	W	E
16-1	1.4		1.4				
16-2b	1.2	1.6	1.2	1.6	0.5		
16-3a	1.2			0.5	1.6		
16-3b	1.2				1.6	0.8	
16-4	1.2			0.5	0.5	1.6	
16-5	1.2			0.5			1
	(1.2 + 0.2S _{cs})			0.5			ρ
	(1.2 + 0.2S _{cs})			0.5			Ω_o
16-6	0.9	1.6				1.6	
16-7	0.9	1.6					1
	(0.9 - 0.2S _{cs})	1.6					ρ
	(0.9 - 0.2S _{cs})	1.6					Ω_o
HOR CANT SEISMIC							-0.2 VERT

ASD (ALLOWABLE STRESS DESIGN)

pg 308 IBC 2009	D	H	F	L	Lr/R	W	E
16-8	1		1				
16-9	1	1	1	1			
16-10	1	1	1		1		
16-11	1	1	1	0.75	0.75		
16-12a	1	1	1			1	
16-12b	1	1	1				0.7
	(1 + 0.14S _{cs})	1	1				0.7p
ASIF = 1.2	(1 + 0.14S _{cs})	1	1				0.7 Ω_o
16-13a	1	1	1	0.75	0.75	0.75	
16-13b	1	1	1	0.75	0.75		0.525
	(1 + 0.10S _{cs})	1	1	0.75	0.75		0.525p
ASIF = 1.2	(1 + 0.10S _{cs})	1	1	0.75	0.75		0.525 Ω_o
16-14	0.6	1				1	
16-15	0.6	1					0.7
	(1 + 0.14S _{cs})	1					0.7p
ASIF = 1.2	(1 + 0.14S _{cs})	1					0.7 Ω_o
HOR CANT SEISMIC							-0.2 VERT

- D - DEAD LOAD
 L - LIVE LOAD
 Lr - ROOF LIVE LOAD
 W - WIND LOAD
 E - SEISMIC LOAD
 H - LATERAL EARTH PRESSURE
 F - WELL DEFINED FLUID
 T - SELF STRAINING FORCES FROM TEMPERATURE CHANGE, CREEP, SHRINKAGE

IF SHEET IS LESS THAN 22" X 34"
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REVISIONS		
No.	Description	Date
1	NOTE CHANGE + STAMP UPDATE	04/30/2019

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I HEREBY CERTIFY THAT THIS PLAN WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION
 DATE: 04/30/2019

Project:
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Title:
 GENERAL NOTES & STRUCTURAL DESIGN CRITERIA

Designed: TG
 Drawn: AM

Checked: TG
 Supv: TG

Scale: AS_SHOWN
 Date: 06/06/2017

Project No. AutoCAD File
 Drawing No.

S0.1

Sheet No. of

EMBEDMENT AND LAP NOTES

ALL CONTINUOUS REINFORCEMENT SHALL BE PLACED USING MINIMUM LAP SPLICE LENGTHS AS SHOWN ON TABLE

PROVIDE STANDARD AC1318 90 OR 180 DEGREE HOOK IF EMBEDMENT IS LESS THAN MINIMUM SHOWN IN TABLE.

REINFORCEMENT PASSING BETWEEN ELEMENTS (CMU/CONCRETE, SLAB/WALL, BEAM/COLUMN etc.) SHALL HAVE MINIMUM EMBEDMENT ON EACH SIDE OF INTERFACE

FOR SLABS, LOCATED BOTTOM BAR SPLICES ABOVE SUPPORTS, TOP BAR SPLICES AT MID THIRD OF SPAN, STAGGER ALL SPLICES BY NO LESS THAN THE REQUIRED MIN SPLICE LENGTH

FOR DOUBLE CURTAIN WALL REINFORCEMENT STAGGER SPLICES OF TWO CURTAINS BY NO LESS THAN REQUIRED MIN SPLICE LENGTH

FOR COLUMNS LOCATE VERTICAL BAR SPLICES AT MIDDLE OF CLEAR HEIGHT BETWEEN STORIES

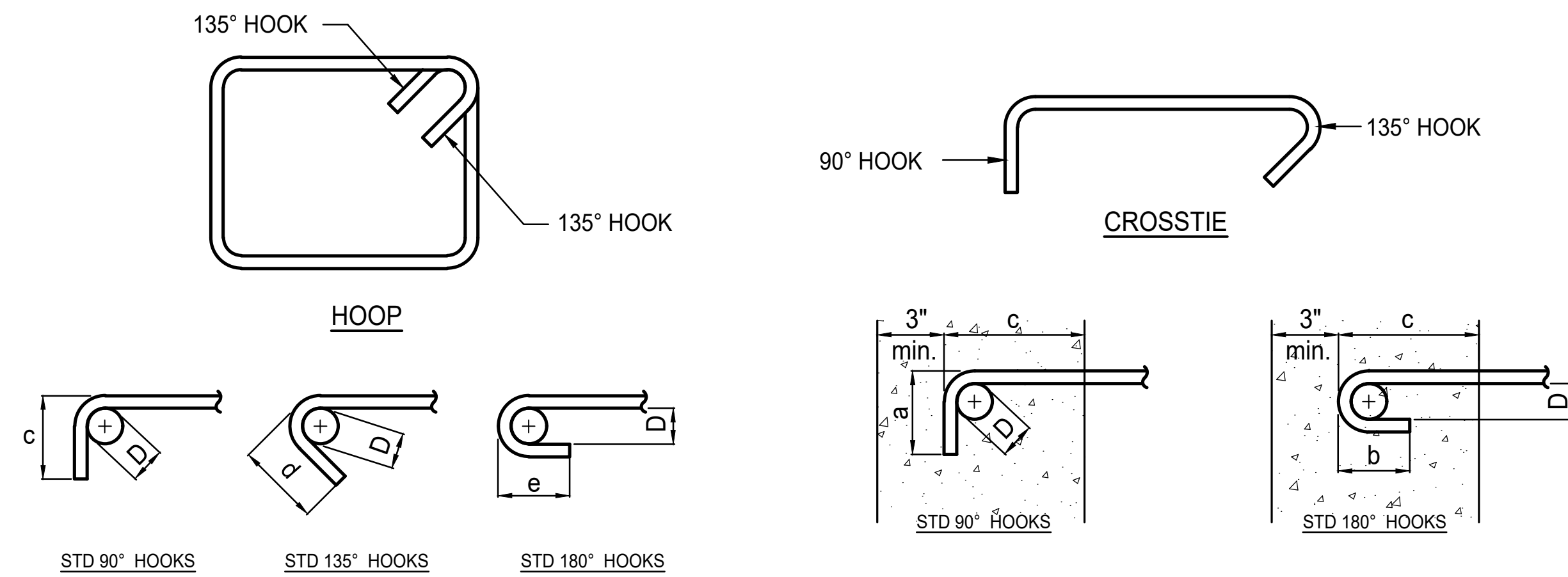
FOR BEAMS LOCATE TOP BAR SPLICES AT MIDSPAN, BOTTOM BAR SPLICES AT THIRD POINTS OF SPAN.

FOR BEAMS AND COLUMNS DO NOT PLACE ANY SPLICES IN BEAM-COLUMN JOINTS

MINIMUM DEVELOPMENT AND SPLICE LENGTHS FOR CMU AND CONCRETE				
BAR SIZE	MINIMUM STRAIGHT EMBEDMENT (INCHES)		MINIMUM LAP REQUIREMENTS (INCHES)	
	TOP *	OTHERS	TOP *	OTHERS
#3	22	16	28	22
#4	28	21	36	28
#5	36	27	46	36
#6	42	32	54	42
#7	48	38	64	48
#8	56	45	72	56
#9	64	49	100	64

* - TOP BARS HAVE MORE THAN 12 INCHES OF FRESH CONCRETE CAST BELOW THEM

BENDS, HOOPS & CROSSTIES



HOOPS & CROSSTIES - STD 90°, 135° & 180° HOOKS				
BAR SIZE	c (MIN)	d (MIN)	e (MIN)	D
#3	5"	5"	5"	1 1/2"
#4	5"	5"	5"	2"
#5	6"	6"	6"	2 1/2"
#6	12"	8"	8"	4 1/2"

PRIMARY REINFORCING STD 90° and 180° HOOK				
BAR SIZE	a (MIN.)	b (MIN.)	c (MIN.)	D
#3	6"	6"	9"	2 1/4"
#4	8"	6"	12"	3"
#5	10"	6"	14"	3 3/4"
#6	12"	6"	18"	4 1/2"
#7	14"	8"	20"	5 1/4"
#8	16"	8"	22"	6"
#9	19"	15"	25"	9"
#10	21"	17"	28"	10"

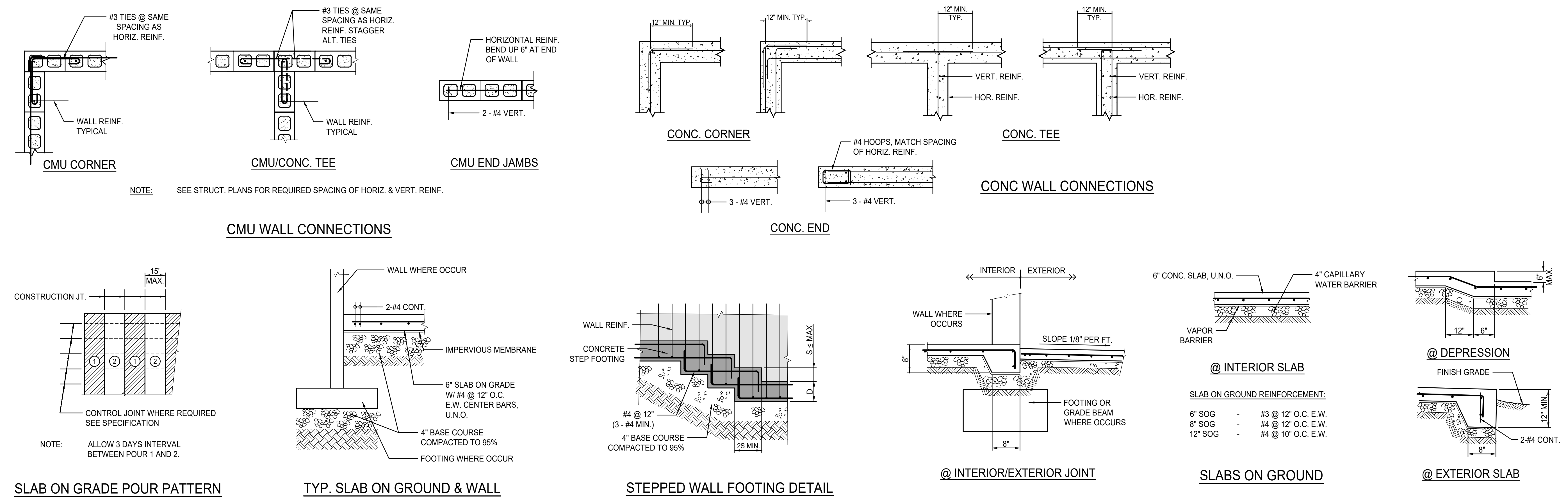
STRUCTURAL ABBREVIATIONS

- | | |
|----------------------------|----------------------------------|
| BOT. BOTTOM | INFO. INFORMATION |
| C.I.P. CAST-IN-PLACE | INT. INTERIOR |
| CJ CONSTRUCTION JOINT | KSF KIPS PER SQUARE FEET |
| CL CENTERLINE | KSI KIPS PER SQUARE INCH |
| CLR. CLEAR(ANCE) | MAX. MAXIMUM |
| CMU CONCRETE MASONRY UNIT | MFR MANUFACTURER |
| CONC. CONCRETE | MIN. MINIMUM |
| CONT. CONTINUOUS | MISC. MISCELLANEOUS |
| DET. DETAIL | N.T.S. NOT TO SCALE |
| DIA. DIAMETER | O.C. ON CENTER |
| DIM. DIMENSION | O.D. OUTSIDE DIAMETER |
| DWG. DRAWING | OPP. OPPOSITE |
| EA. EACH | PL. PLATE |
| E.F. EACH FACE | PSI POUNDS PER SQUARE INCH |
| E.J. EXPANSION JOINT | PSF POUNDS PER SQUARE FEET |
| EQ. EQUAL | REINF. REINFORCED, REINFORCEMENT |
| EQUIPT. EQUIPMENT | SHT. SHEET |
| E.S. EACH SIDE | S.O.G. SLAB ON GRADE |
| E.W. EACH WAY | SPECS. SPECIFICATION |
| EXP. EXPANSION | SQ. SQUARE |
| EXIST. EXISTING | STRUCT. STRUCTURAL |
| EXT. EXTERIOR | t THICKNESS |
| FDN FOUNDATION | T&B TOP & BOTTOM |
| FFE FINISH FLOOR ELEVATION | TM TOP MOST BARS |
| FL. FLOOR | BM BOTTOM MOST BARS |
| F.O.C. FACE OF CONCRETE | T.O.F. TOP OF FOOTING |
| FT. FOOT OR FEET | T.O.S. TOP OF SLAB, TOP OF STEEL |
| FTG. FOOTING | T.O.W. TOP OF WALL |
| GA. GAUGE | TYP. TYPICAL |
| GALV. GALVANIZED | U.N.O. UNLESS NOTED OTHERWISE |
| HORIZ. HORIZONTAL | VERT. VERTICAL |
| I.D. INSIDE DIAMETER | W/ WITH |

REVISIONS		
No.	Description	Date
1	STAMP UPDATE	04/30/2019

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TYPICAL DETAILS



Professional Engineer Seal for G. U. A. M. No. 1781 (Structural), Exp. 04/30/2020. Date: 04/30/2019.

Project: GMH

Title: EMBEDMENT & LAP NOTES AND TYPICAL DETAILS

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STATEMENT OF SPECIAL INSPECTIONS

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE 2009 INTERNATIONAL BUILDING CODE (2009IBC).

THIS STATEMENT OF SPECIAL INSPECTIONS ENCOMPASS THE FOLLOWING DISCIPLINES:

- STRUCTURAL SPECIAL INSPECTIONS PER 1704 AND 1705
- STRUCTURAL SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE
- STRUCTURAL SPECIAL INSPECTIONS FOR WIND RESISTANCE

THIS SCHEDULE OF SPECIAL INSPECTIONS SUMMARIZES THE SPECIAL INSPECTIONS AND TEST REQUIRED. SPECIAL INSPECTORS WILL REFER TO THE APPROVED PLANS AND SPECIFICATIONS FOR DETAILED SPECIAL REQUIREMENTS. ANY ADDITIONAL TESTS AND INSPECTIONS REQUIRED BY THE APPROVED PLANS AND SPECIFICATIONS WILL ALSO BE PERFORMED.

THE SPECIAL INSPECTIONS IDENTIFIED ARE IN ADDITION TO THOSE REQUIRED BY OTHER SECTIONS OF THE BUILDING CODE.

THE SPECIAL INSPECTIONS IDENTIFIED ARE IN ADDITION TO THOSE REQUIRED BY OTHER SECTIONS OF THE BUILDING CODE.

THE SPECIAL INSPECTION COORDINATOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE COFR AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITIES.

INTERIM REPORTS SHALL BE SUBMITTED TO THE OWNER AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH THE SECTION 1704.1.2.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS, TESTING AND CORRECTIONS OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PER SECTION 1704.1.2. THE FINAL REPORT WILL DOCUMENT THE REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF DISCREPANCIES NOTED IN INSPECTIONS.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

THE CONTRACTOR IS REQUIRED TO COORDINATE ALL INSPECTIONS. THE CONTRACTOR SHALL NOTIFY THE COFR AND THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO ANY SPECIAL INSPECTIONS THAT ARE REQUIRED. THE CONTRACTOR SHALL NOTIFY THE OWNER AND THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO ANY CONCRETE TO BE POURED.

THE INSPECTORS AND TESTING AGENCIES SHALL BE ENGAGED BY THE OWNER, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED PER SECTION 1704.1. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE COFR PRIOR TO COMMENCING WORK. IF APPROPRIATE AGENTS ARE NOTED AS "TO BE DETERMINED (TBD)", THE COFR IS RESPONSIBLE TO COORDINATE THE ASSEMBLY OF A SPECIAL INSPECTION TEAM.

SPECIFICALLY INSPECTED WORK THAT IS INSTALLED WITHOUT THE APPROVAL OF THE OWNER IS SUBJECT TO REMOVAL.

CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED, WHEN WORK IN MORE THAN CATEGORY OF WORK REQUIRING SPECIAL INSPECTION IS TO BE PERFORMED SIMULTANEOUSLY, OR THE GEOGRAPHIC LOCATION OF THE WORK IS SUCH THAT IT CANNOT BE CONTINUOUSLY OBSERVED. IT IS THE OWNERS RESPONSIBILITY TO EMPLOY A SUFFICIENT NUMBER OF INSPECTORS TO ASSURE THAT ALL THE WORK IS INSPECTED IN ACCORDANCE WITH THE PROVISIONS OF THE BUILDING CODE.

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION AND TESTING ACTIVITIES ARE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED IF REQUESTED.

KEY FOR MINIMUM QUALIFICATIONS OF INSPECTION AGENTS:

WHEN THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE DEEMS IT APPROPRIATE THAT THE INDIVIDUAL PERFORMING A STIPULATED TEST OR INSPECTION HAVE A SPECIFIC CERTIFICATION OR LICENSE AS INDICATED BELOW, SUCH DESIGNATION SHALL APPEAR BELOW THE AGENCY NUMBER ON THE SCHEDULE.

PE/SE	STRUCTURAL ENGINEER	A LICENSED SE OR PE SPECIALIZING IN THE DESIGN OF BUILDING STRUCTURES.
PE/GE	GEOTECHNICAL ENGINEER	A LICENSED GE OR PE SPECIALIZING IN SOIL MECHANICS AND FOUNDATIONS
EIT	ENGINEER-IN-TRAINING	A GRADUATE ENGINEER WHO HAS PASSED THE FUNDAMENTALS OF ENGINEERING EXAMINATION

AMERICAN CONCRETE INSTITUTE (ACI) CERTIFICATION

ACI-CFTT	CONCRETE FIELD TESTING TECHNICIAN - GRADE 1
ACI-CCI	CONCRETE CONSTRUCTION INSPECTOR
ACI-LTT	LABORATORY TESTING TECHNICIAN - GRADE 1&2
ACI-STT	STRENGTH TESTING TECHNICIAN

AMERICAN WELDING SOCIETY (AWS) CERTIFICATION

AWS-CWI	CERTIFIED WELDING INSPECTOR
AWS/AISC-SSI	CERTIFIED STRUCTURAL STEEL INSPECTOR

INTERNATIONAL CODE COUNCIL (ICC) CERTIFICATION

ICC-SMSI	STRUCTURAL MASONRY SPECIAL INSPECTOR
ICC-SWSI	STRUCTURAL STEEL AND WELDING SPECIAL INSPECTOR
ICC-SFSI	SPRAY-APPLIED FIREPROOFING SPECIAL INSPECTOR
ICC-RCSI	REINFORCED CONCRETE SPECIAL INSPECTOR

SCHEDULE OF INSPECTIONS

<input checked="" type="checkbox"/> SOILS AND FOUNDATIONS	<input type="checkbox"/> WOOD CONSTRUCTION
<input checked="" type="checkbox"/> CAST-IN-PLACE CONCRETE	<input type="checkbox"/> MECHANICAL & ELECTRICAL SYSTEMS
<input type="checkbox"/> PRECAST CONCRETE	<input type="checkbox"/> ARCHITECTURAL SYSTEMS
<input checked="" type="checkbox"/> MASONRY LEVEL 1	<input checked="" type="checkbox"/> STRUCTURAL STEEL
<input checked="" type="checkbox"/> MASONRY LEVEL 2	<input checked="" type="checkbox"/> COLD-FORMED STEEL FRAMING

DESIGNATED SEISMIC FORCE RESISTING SYSTEMS

SEISMIC DESIGN CATEGORY 'D'

DESCRIPTION OF SEISMIC-FORCE-RESISTING SYSTEM AND DESIGNATED SEISMIC SYSTEMS SUBJECT TO SPECIAL INSPECTIONS AS PER SECTION 1705.3:

GMH - SPECIAL REINFORCED CONCRETE SHEAR WALLS

THE EXTENT OF THE SEISMIC-FORCE-RESISTING SYSTEM IS DEFINED IN MORE DETAIL IN THE CONSTRUCTION DOCUMENTS.

ITEM 7: 1707.7 - MECHANICAL AND ELECTRICAL COMPONENTS

SCOPE:	PERIODIC	CONTINUOUS
A. INSPECT ANCHORAGE OF ELECTRICAL EQUIPMENT FOR EMERGENCY OR STAND-BY POWER SYSTEMS.	X	
B. INSPECT ANCHORAGE OF NON-EMERGENCY ELECTRICAL EQUIPMENT.	X	
C. INSPECT INSTALLATION OF PIPING SYSTEMS AND ASSOCIATED MECHANICAL UNITS CARRYING FLAMMABLE, COMBUSTIBLE, OR HIGHLY TOXIC CONTENTS.	X	
D. INSPECT INSTALLATION OF HVAC DUCTWORK THAT CONTAINS HAZARDOUS MATERIALS.	X	
E. INSPECT INSTALLATION OF VIBRATION OF ISOLATION SYSTEMS WHERE REQUIRED BY SECTION 1707.8.	X	

TABLE 1704.7 - INSPECTION OF SOILS

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	AGENCY # (QUALIF.)
1	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIRED BEARING CAPACITY.	X		PE/GE
2	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	X		PE/GE
3	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS. PERFORM SIEVE TESTS (ASTM D422 & D1140); ATTERBERG LIMIT TEST (ASTM D4318) AND MODIFIED PROCTOR TESTS (ASTM D1557) OF EACH SOURCE OF FILL MATERIAL.	X		PE/GE
4	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL. TEST DENSITY OF EACH LIFT OF FILL BY NUCLEAR METHODS (ASTM D6938) OR SAND CONE (ASTM D1556). VERIFY EXTENT AND SLOPE OF FILL PLACEMENT. VERIFY COMPACTION OF FILL AND BACKFILL MATERIAL TO 95 PERCENT OF ASTM D 1557. TEST EACH LIFT AT RANDOMLY SELECTED LOCATIONS EVERY 1000 SQUARE FEET OF FILL OR 50 LINEAR FOOT OF WALL OR CONTINUOUS FOOTING, WHICHEVER IS GREATER. PERFORM A MINIMUM OF ONE TEST PER ISOLATED FOOTING. PERFORM 3 TEST MINIMUM PER LIFT.		X	PE/GE
5	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	X		PE/GE

NOTES: SEE GENERAL STRUCTURAL NOTES AND/OR BID DOCUMENTS FOR REFERENCE GEOTECHNICAL REPORT.

TABLE 1704.4 - CONCRETE

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	AGENCY # (QUALIF.)
1	INSPECTION OF REINFORCING STEEL AND PLACEMENT.	X		ACI-CCI, ICC-RCSI
2	INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B.	X		ACI-CCI, ICC-RCSI
3	INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.		X	ACI-CFI, ICC-RCSI
4	INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.	X		ACI-CFTT, ACI-STT
5	VERIFYING USE OF REQUIRED DESIGN MIX.	X		ACI-CCI, ICC-RCSI
6	AT TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS AND DETERMINE THE TEMPERATURE OF THE CONCRETE.		X	ACI-CCI, ICC-RCSI
7	INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.		X	ACI-CCI, ICC-RCSI

TABLE 1704.4 - CONCRETE

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	AGENCY # (QUALIF.)
8	INSPECTION OF MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	X		ACI-CCI, ICC-RCSI
9	INSPECTION OF PRESTRESSED CONCRETE. SCOPE: A. APPLICATION OF PRESTRESSING FORCES. B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM.		X	ACI-CCI, ICC-RCSI
10	ERECTION OF PRECAST CONCRETE MEMBERS.	X		ACI-CCI, ICC-RCSI
11	VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDON IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	X		ACI-CFTT, ACI-STT
12	INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	X		ACI-CCI, ICC-RCSI

TABLE 1704.3 - STEEL

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	AGENCY # (QUALIF.)
1	TABLE 1704.3 - MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS. SCOPE: A. IDENTIFICATION MARKINGS TO CONFORM TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS. B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	X		AWS/AISC-SSI, ICC-SWSI
2	TABLE 1704.3 - INSPECTION OF HIGH-STRENGTH BOLTING. SCOPE: A. SNUG-TIGHT JOINTS B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION. C. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING, OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	X	AWS/AISC-SSI, ICC-SWSI
3	TABLE 1704.3 - MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK. SCOPE: A. IDENTIFICATION MARKINGS TO CONFORM TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED DOCUMENTS. B. MANUFACTURER'S MILL TEST REPORTS	X	X	PE/SE
4	TABLE 1704.3 - MATERIAL VERIFICATION OF WELD FILLER MATERIALS. SCOPE: A. IDENTIFICATION MARKINGS TO CONFORM TO CONFORM AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DRAWINGS. B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	X	X	AWS-CWI
5	TABLE 1704.3 - INSPECTION OF WELDING. SCOPE: A. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK. 1. COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS. 2. MULTIPASS FILLET WELDS 3. SINGLE PASS FILLET WELDS > 5/16" 4. PLUG AND SLOT WELDS 5. SINGLE PASS FILLET WELDS <= 5/16" 6. FLOOR AND ROOF DECK WELDS B. REINFORCING STEEL 1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706. 2. REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS, AND SHEAR REINFORCEMENT. 3. SHEAR REINFORCEMENT 4. OTHER REINFORCING STEEL	X	X	AWS-CWI

TABLE 1704.3 - STEEL

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	AGENCY # (QUALIF.)
6	TABLE 1704.3 - INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS. SCOPE: A. DETAILS SUCH AS BRACING AND STIFFENING B. MEMBER LOCATIONS C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION	X		PE/SE
7	SECTION 1704.3 - WELDED STUDS WHEN USED FOR STRUCTURAL DIAPHRAGMS	X		AWS-CWI, ASNT
8	SECTION 1704.3 - WELDED COLD-FORMED SHEET STEEL FRAMING MEMBERS	X		AWS-CWI, ASNT
9	SECTION 1704.3 - WELDING OD STAIRS AND RAILING SYSTEMS	X		AWS-CWI, ASNT
10	SECTION 1704.2.1 - INSPECT FABRICATOR'S FABRICATION AND QUALITY CONTROL PROCEDURES	X		AWS-CWI, ASNT

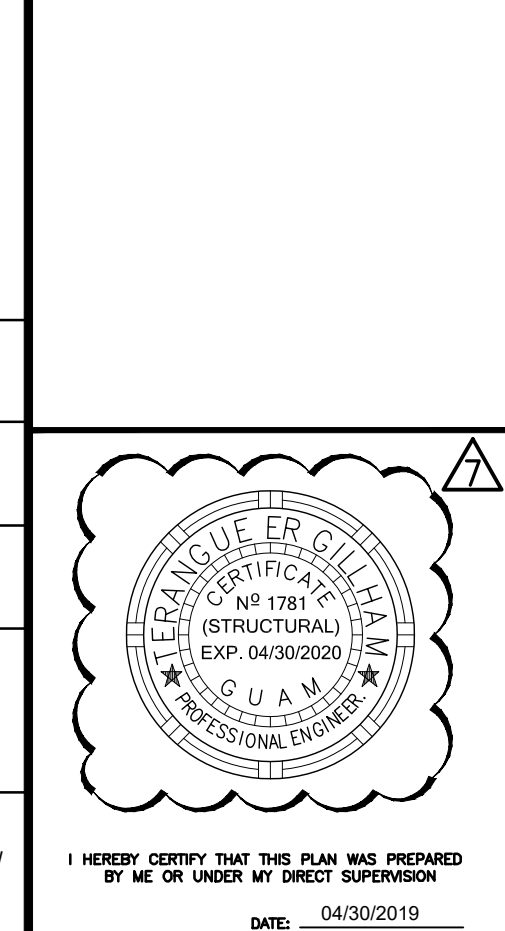
TABLE 1704.5.1 - MASONRY

ITEM NO.	DESCRIPTION	PERIODIC	CONTINUOUS	REFERENCE
1	COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	X		TMS 602/ACI 530.1/ASCE 6 ⁹
2	VERIFICATION OF F_{pu} AND F_{pu} PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE.	X		TMS 602/ACI 530.1/ASCE 6 ⁹
3	VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT		X	TMS 602/ACI 530.1/ASCE 6 ⁹
4	AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE: A. PROPORTIONS OF SITE-PREPARED MORTAR. B. CONSTRUCTION OF MORTAR JOINTS. C. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES. D. PRESTRESSING TECHNIQUE E. GRADE AND SIZE PRESTRESSING TENDONS AND ANCHORAGES	X	X	TMS 602/ACI 530.1/ASCE 6 ⁹
5	DURING CONSTRUCTION THE INSPECTION PROGRAM SHALL VERIFY: A. SIZE & LOCATION OF STRUCTURAL ELEMENTS. B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION. C. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT, ANCHOR BOLTS, PRE-STRESSING TENDONS AND ANCHORAGES. D. WELDING OF REINFORCING BARS. E. PREPARATION CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°) F. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	X	X	TMS 602/ACI 530.1/ASCE 6 ⁹
6	PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE: A. GROUT SPACE IS CLEAN. B. PLACEMENT OF REINFORCEMENT AND CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES. C. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS. D. CONSTRUCTION OF MORTAR JOINTS.	X		TMS 602/ACI 530.1/ASCE 6 ⁹
7	GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE: A. GROUTING OF PRESTRESSING BONDED TENDONS.		X	TMS 602/ACI 530.1/ASCE 6 ⁹
8	PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	X		TMS 602/ACI 530.1/ASCE 6 ⁹ /IBC 2009

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Project: GMH

Title: STATEMENT OF SPECIAL INSPECTION

Designed: TG

Drawn: AM

Checked: TG

Supv: TG

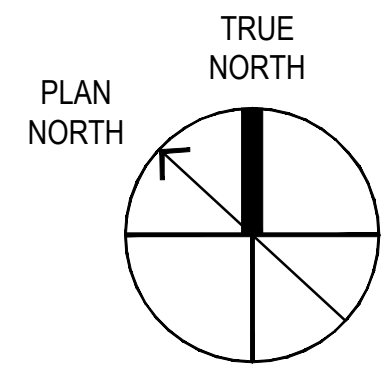
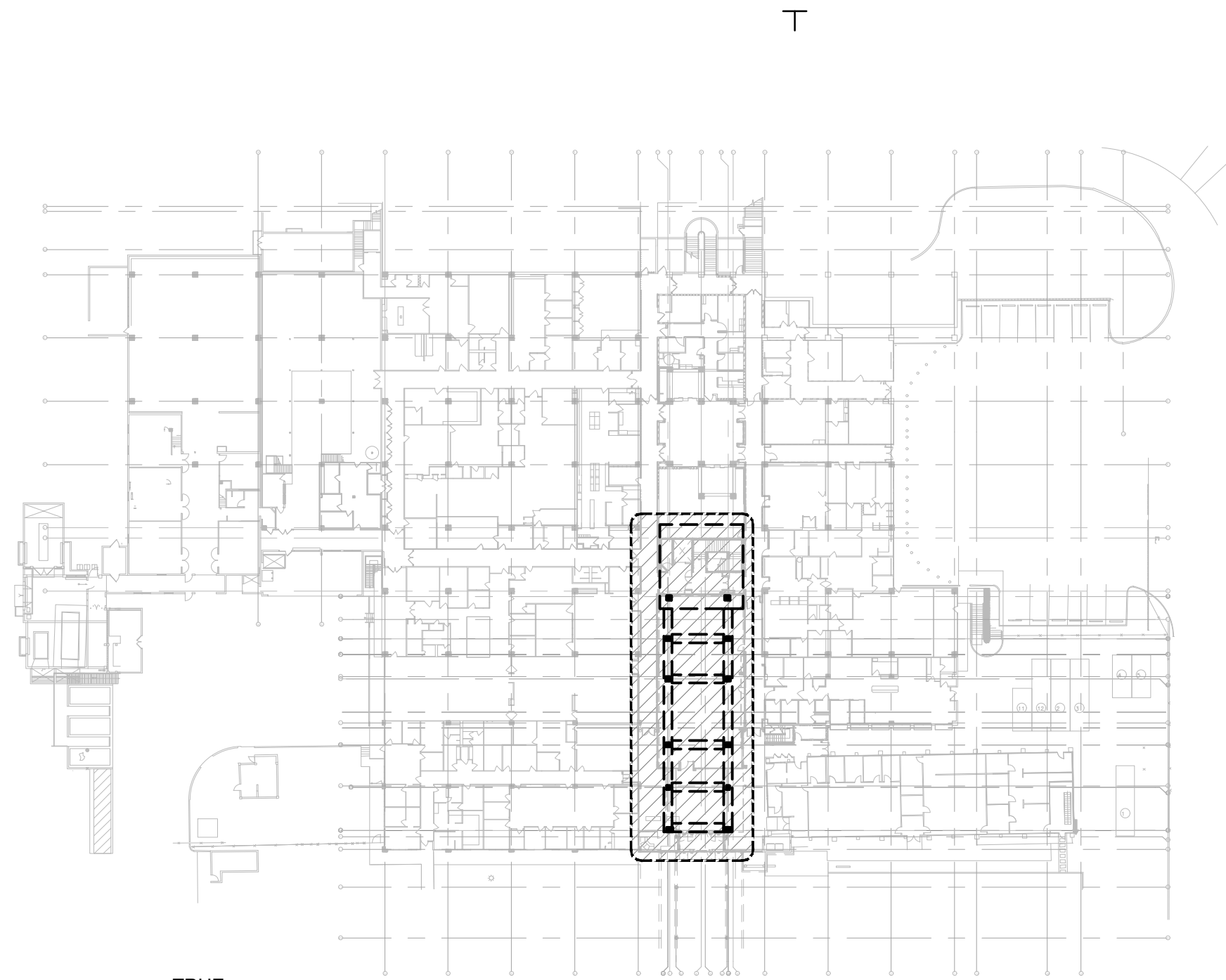
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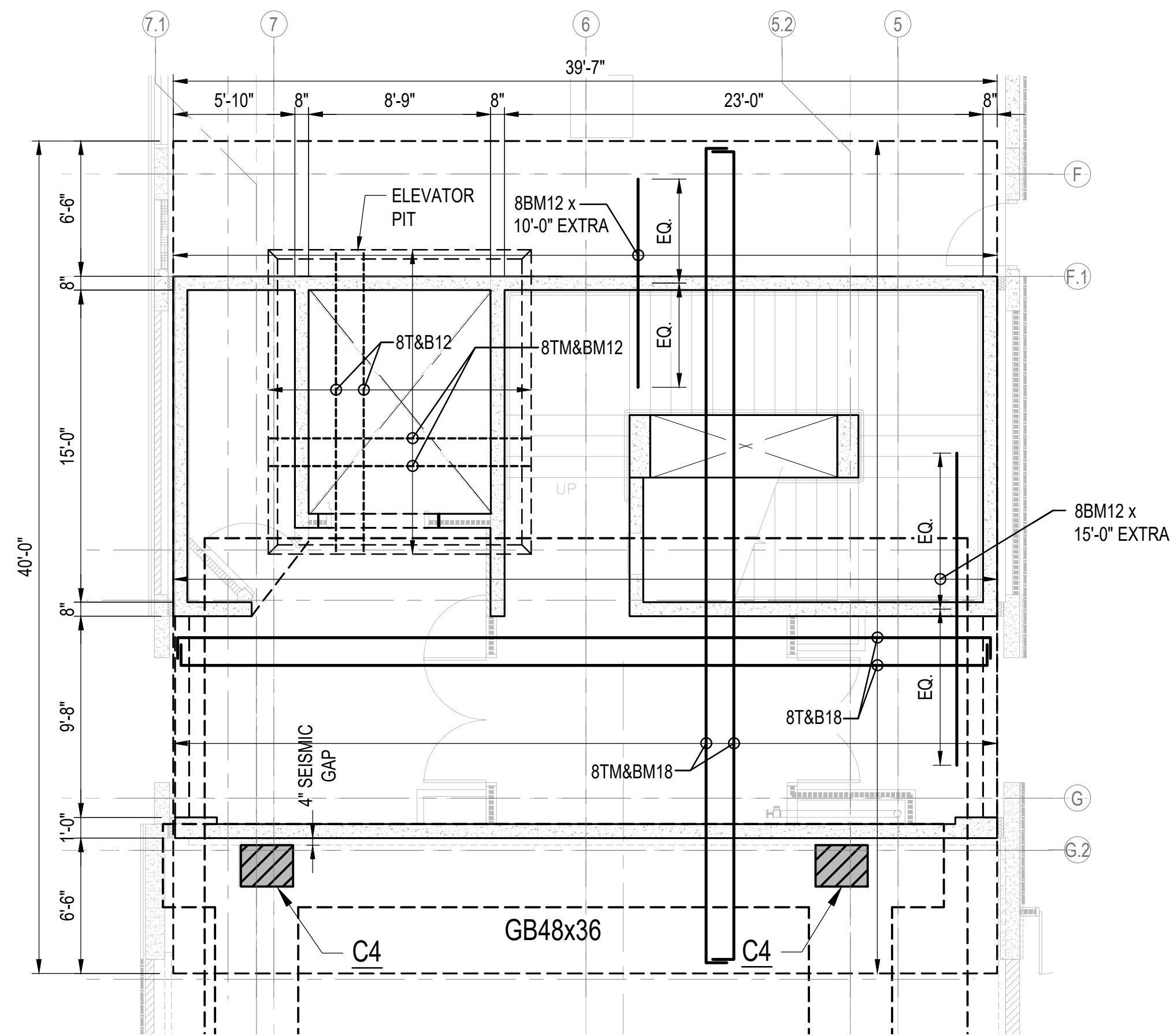
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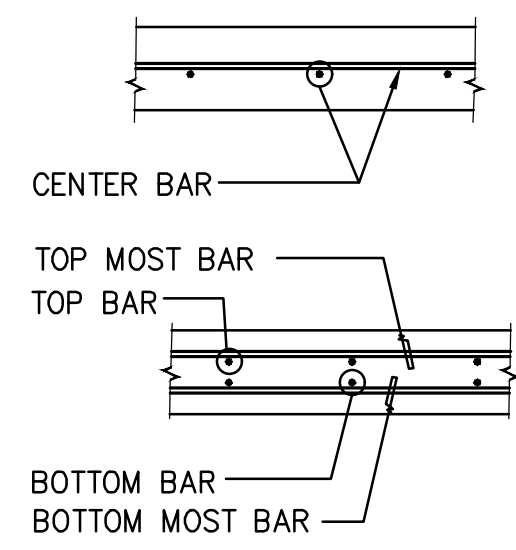
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KEYPLAN



2 MAT SLAB ENLARGED PLAN
SCALE 3/16" = 1'-0"



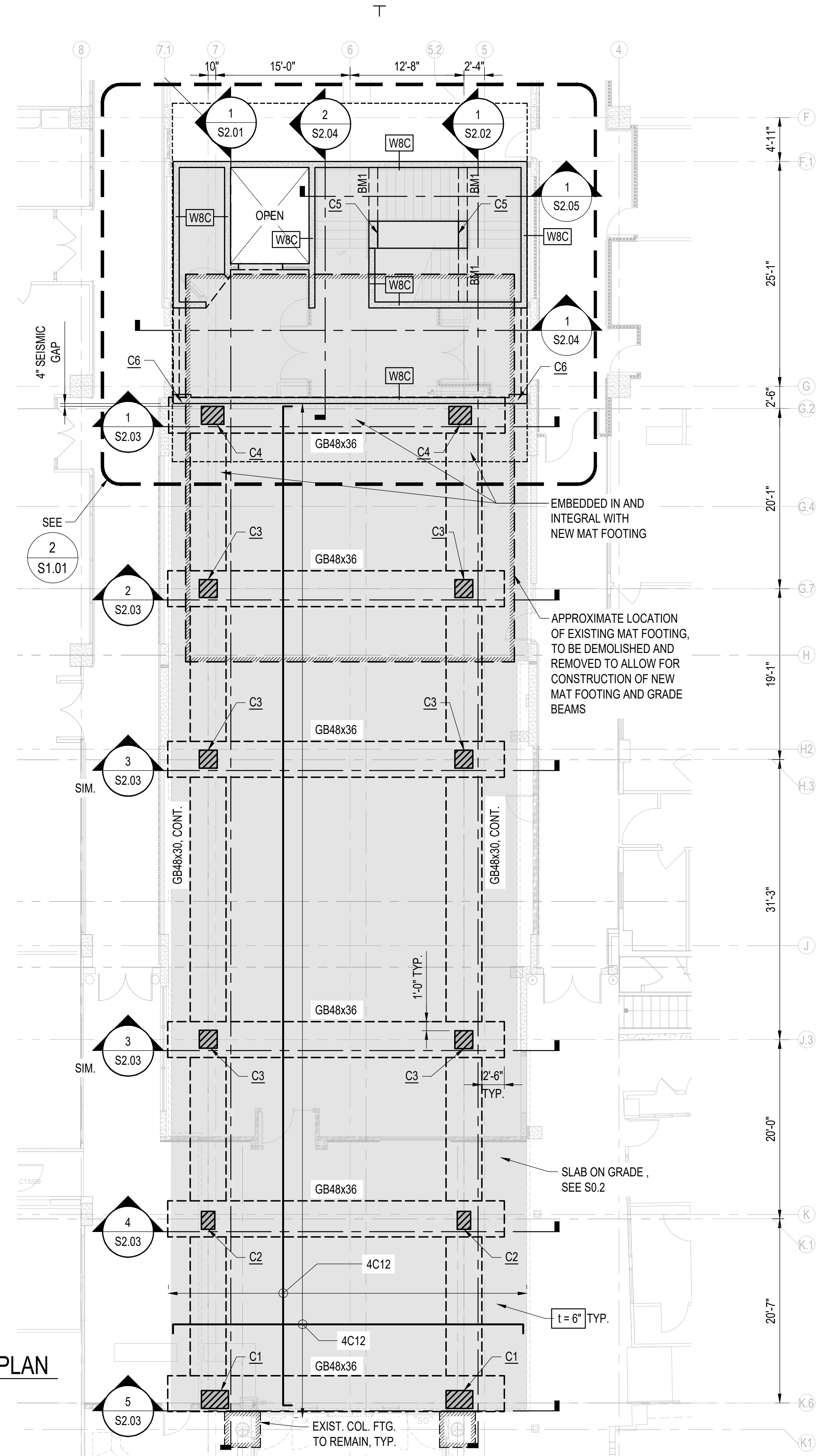
NUMBER OF BARS REQUIRED (WHERE OCCURS)
 BAR SIZE
 T = TOP BAR
 B = BOTTOM BAR
 TM = TOP MOST BAR
 BM = BOTTOM MOST BAR
 C = CENTER BAR
 ON-CENTER SPACING (LAST 2 DIGITS), INCHES
 LENGTH OF BARS (WHERE OCCURS)

SLAB REINF. DETAIL

LEGEND:

- GBxx ----- GRADE BEAM DESIGNATION, SEE SHEET S2.01
- C1 ----- COLUMN DESIGNATION, SEE SHEET S3.01
- W8C ----- WALL DESIGNATION, SEE SHEET S3.01

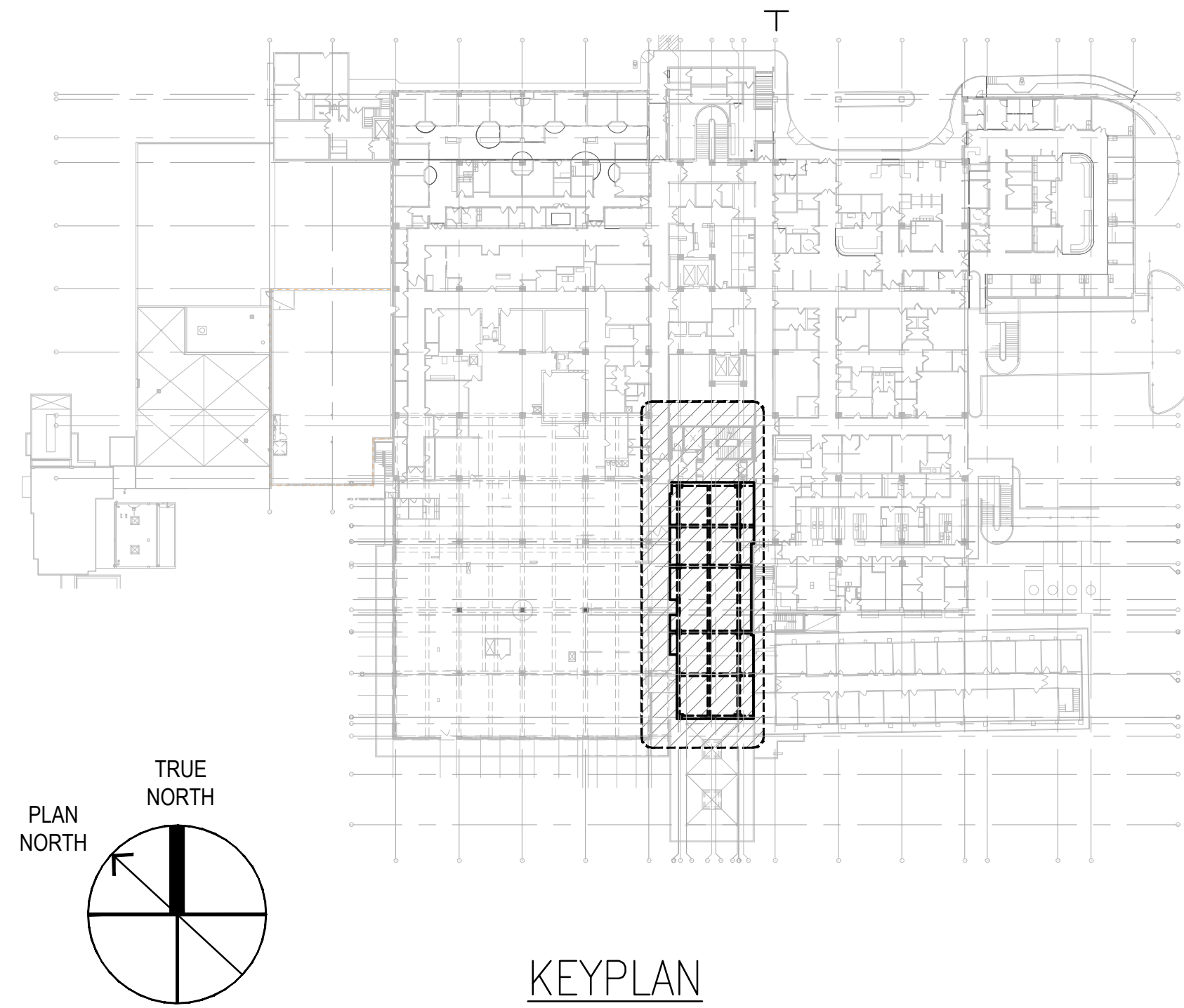
1 FOUNDATION PLAN
SCALE 1/8" = 1'-0"



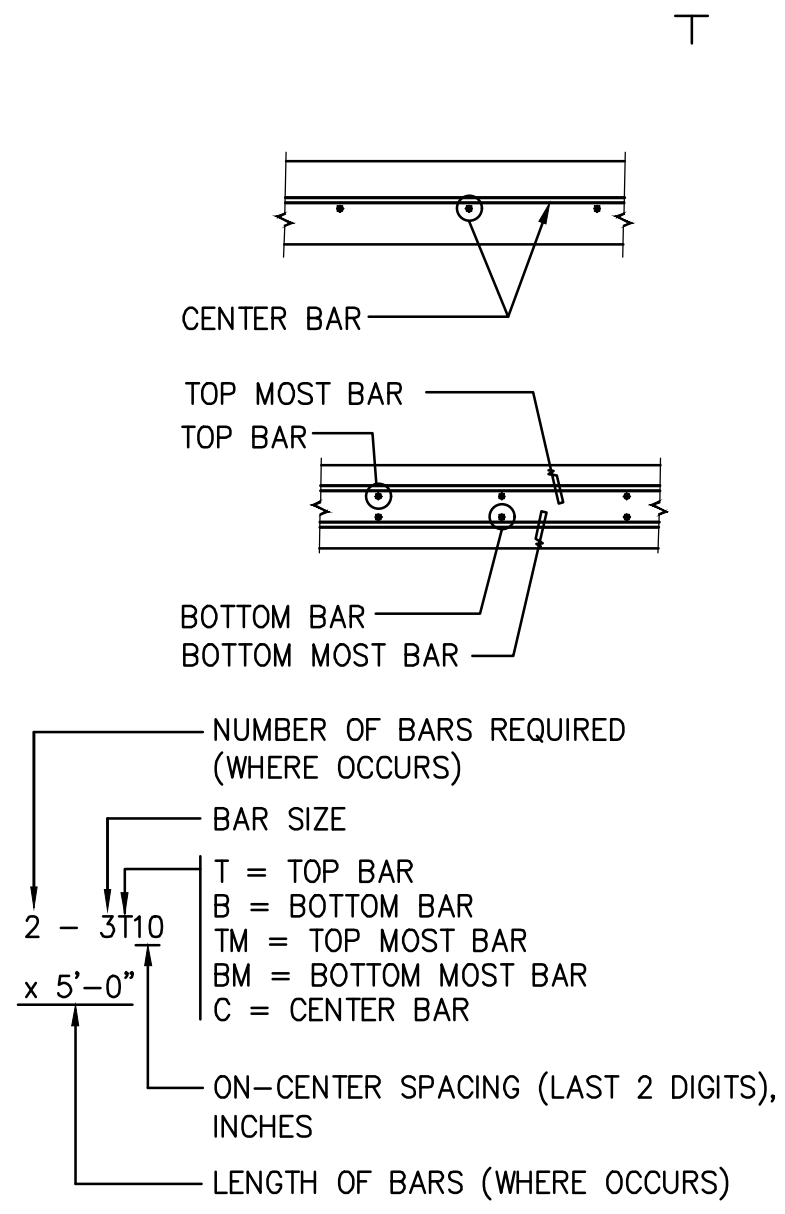
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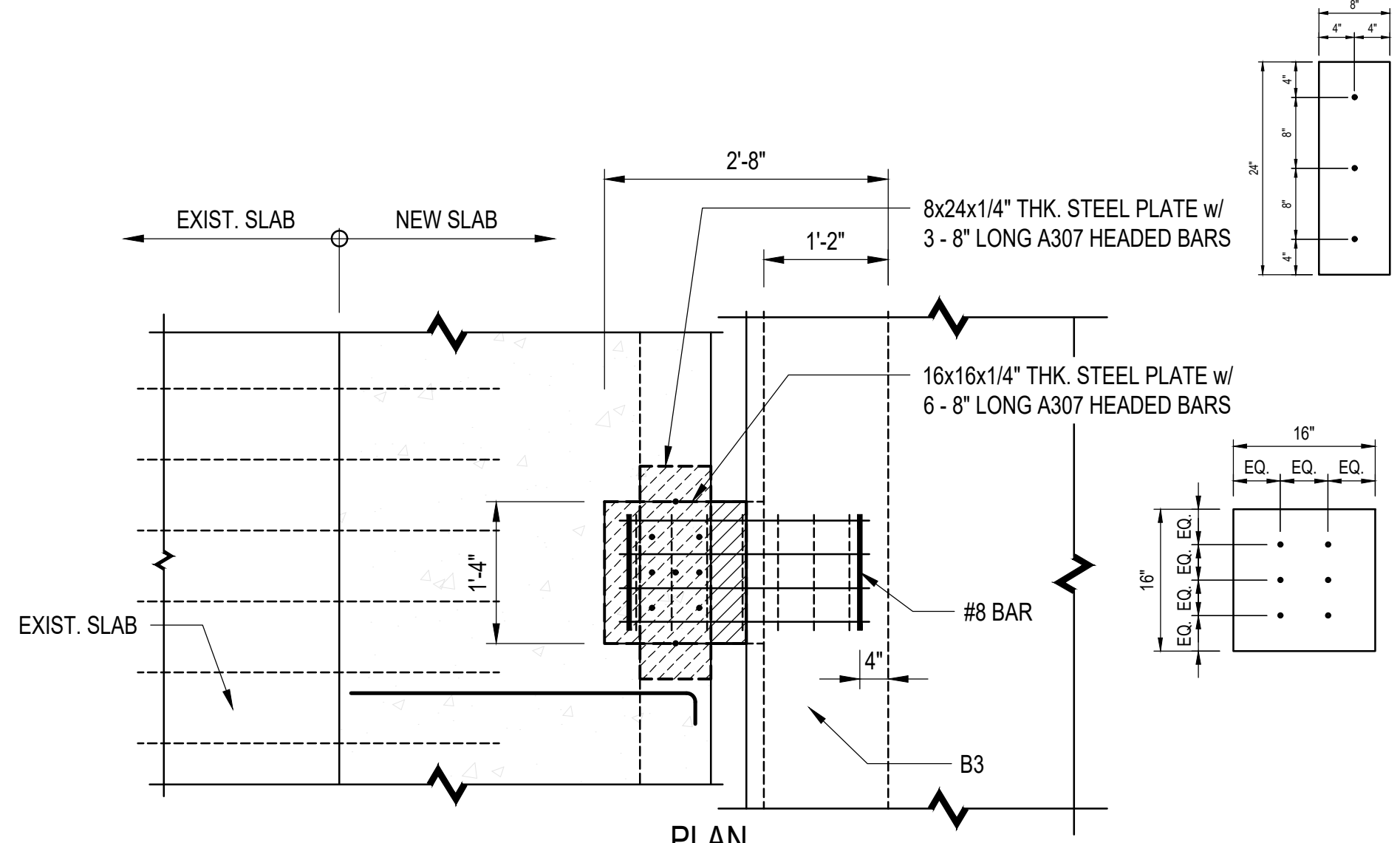
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Project:	GMH
Title:	FOUNDATION PLAN
Designed:	TG
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Supv:	TG
Scale:	AS_SHOWN
Date:	06/06/2017
Project No.:	AutoCAD File
Drawing No.:	S1.01
Sheet No.:	of



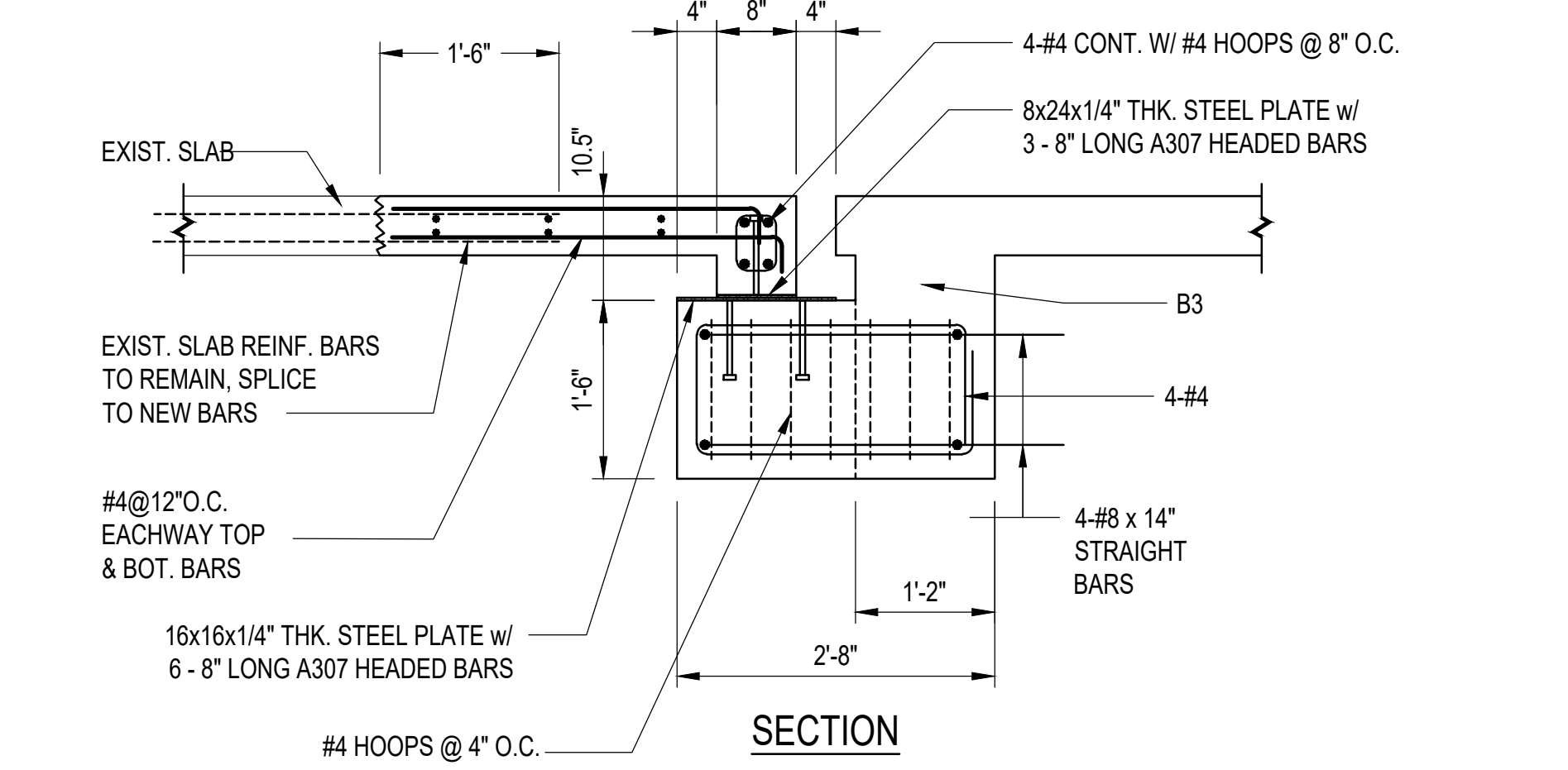
KEYPLAN



SLAB REINF. DETAIL



PLAN

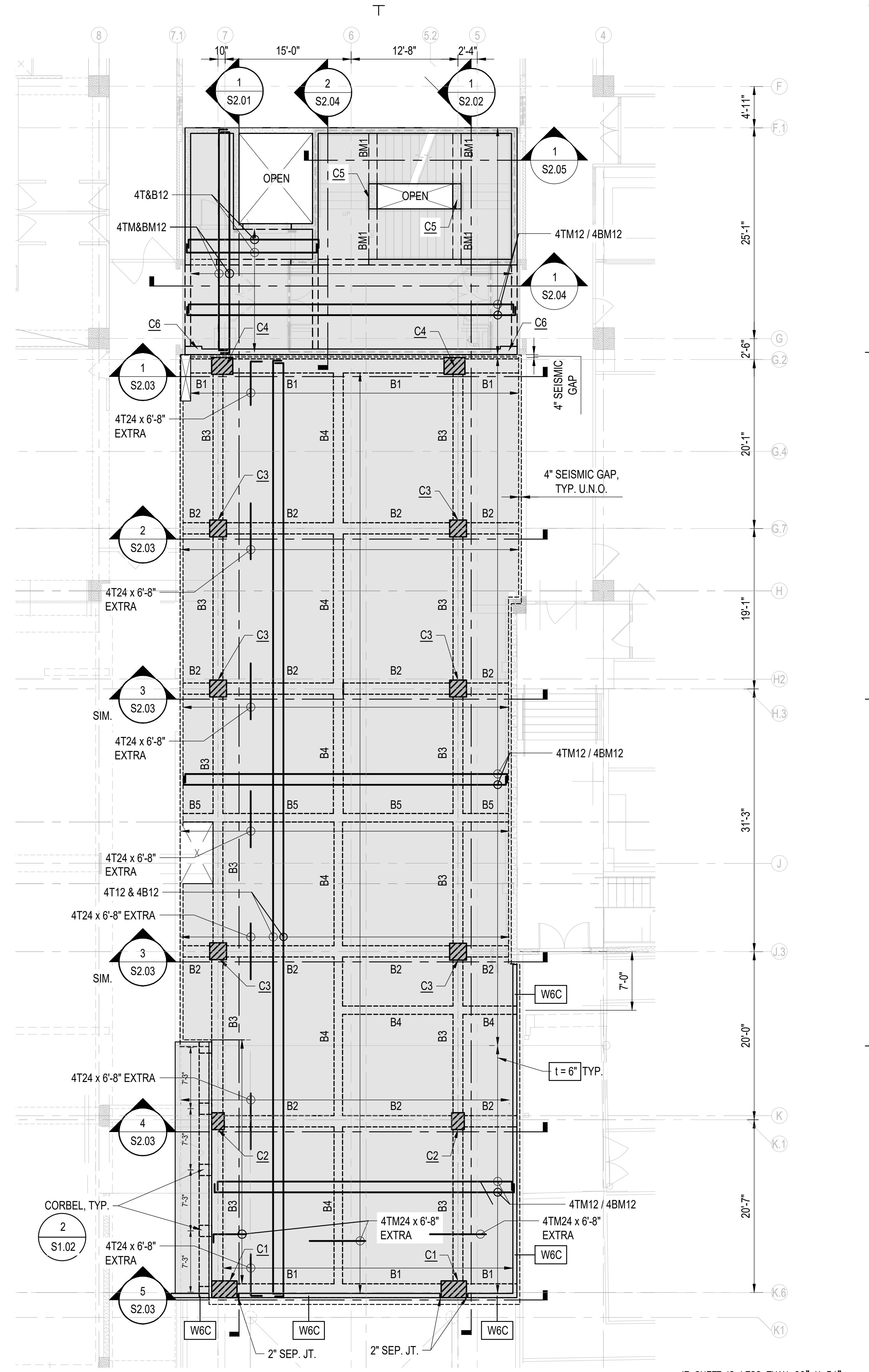


SECTION

2 CORBEL DETAIL
S1.02 | S1.02 SCALE 3/4" = 1'-0"

- LEGEND:
- C1 ----- COLUMN DESIGNATION, SEE SHEET S3.01
 - B1 ----- BEAM DESIGNATION, SEE SHEET S3.01
 - W6C ----- WALL DESIGNATION, SEE SHEET S3.01
 - t=6" ----- SLAB THICKNESS

1 2ND LEVEL FRAMING PLAN
S1.02 | S1.02 SCALE 1/8" = 1'-0"



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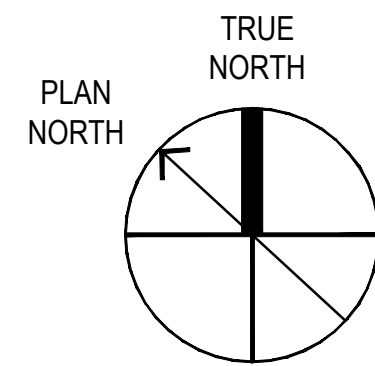
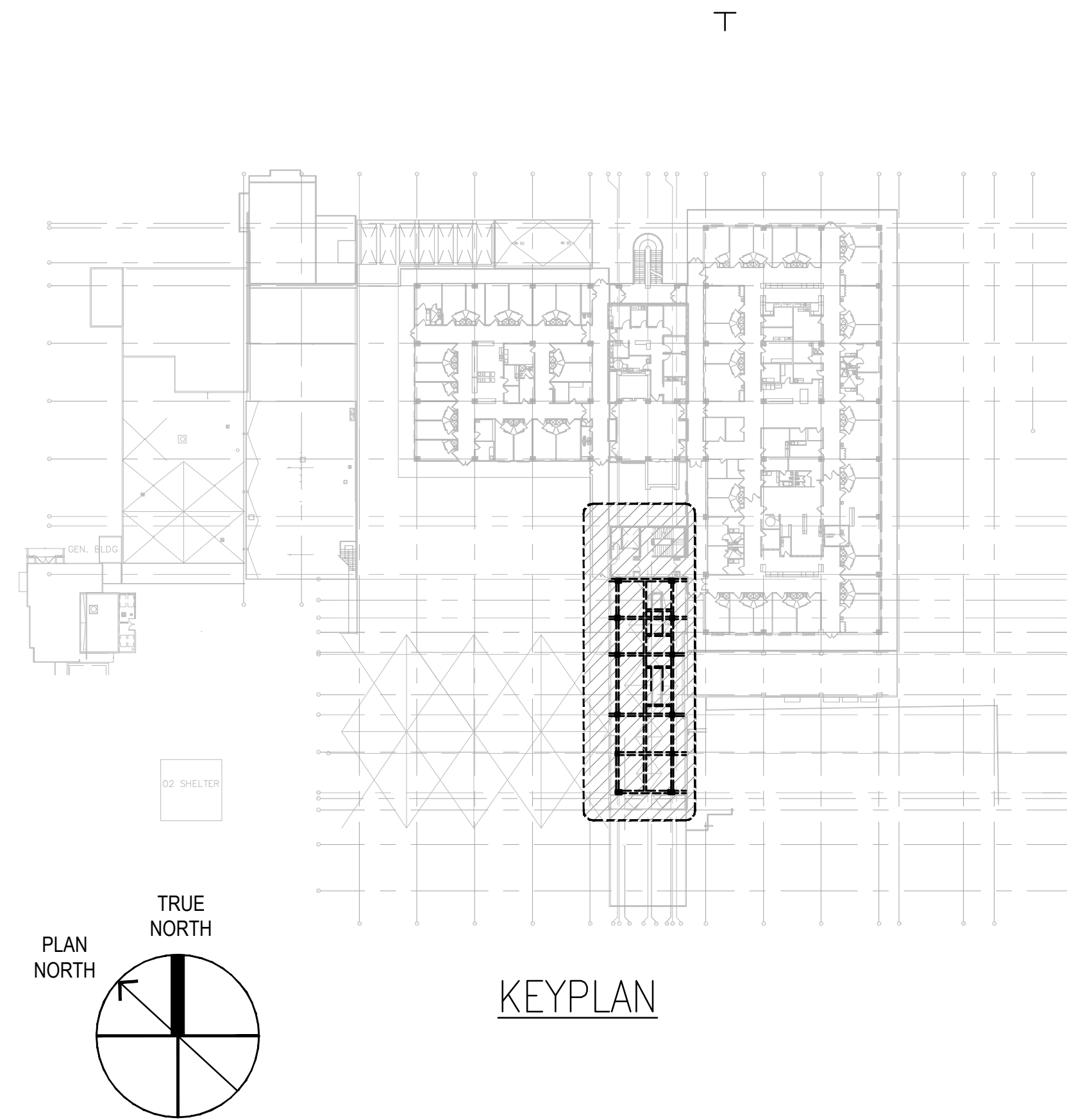
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Title: 2ND LEVEL FRAMING PLAN

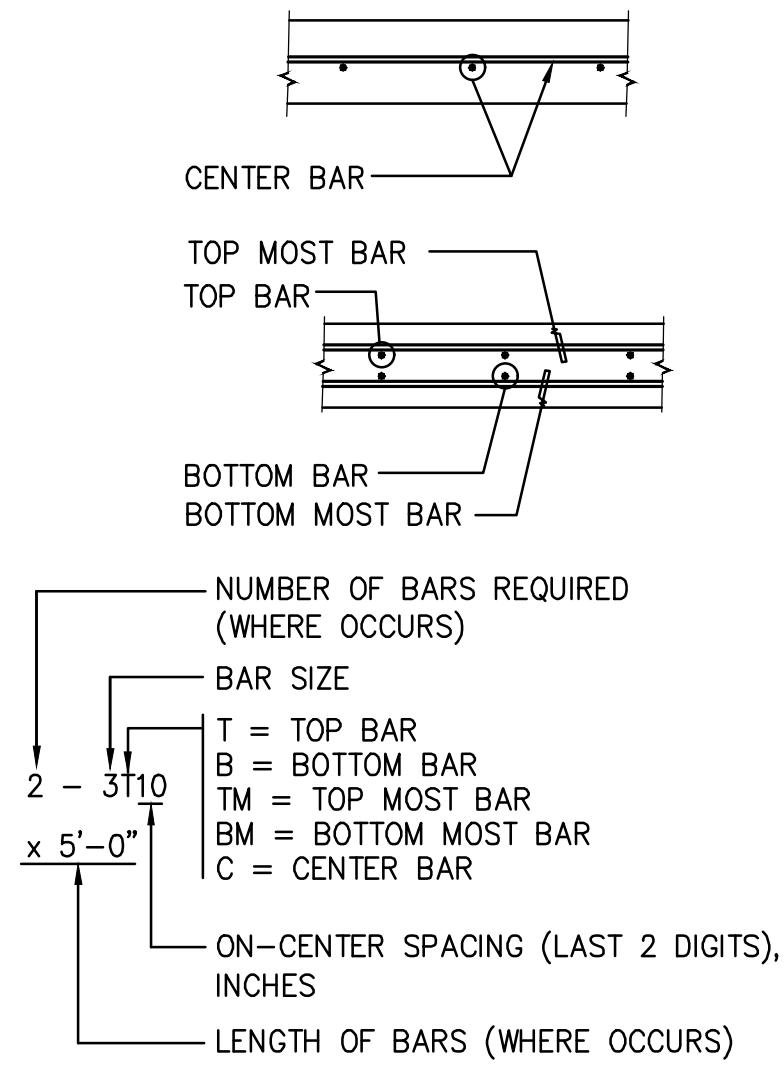
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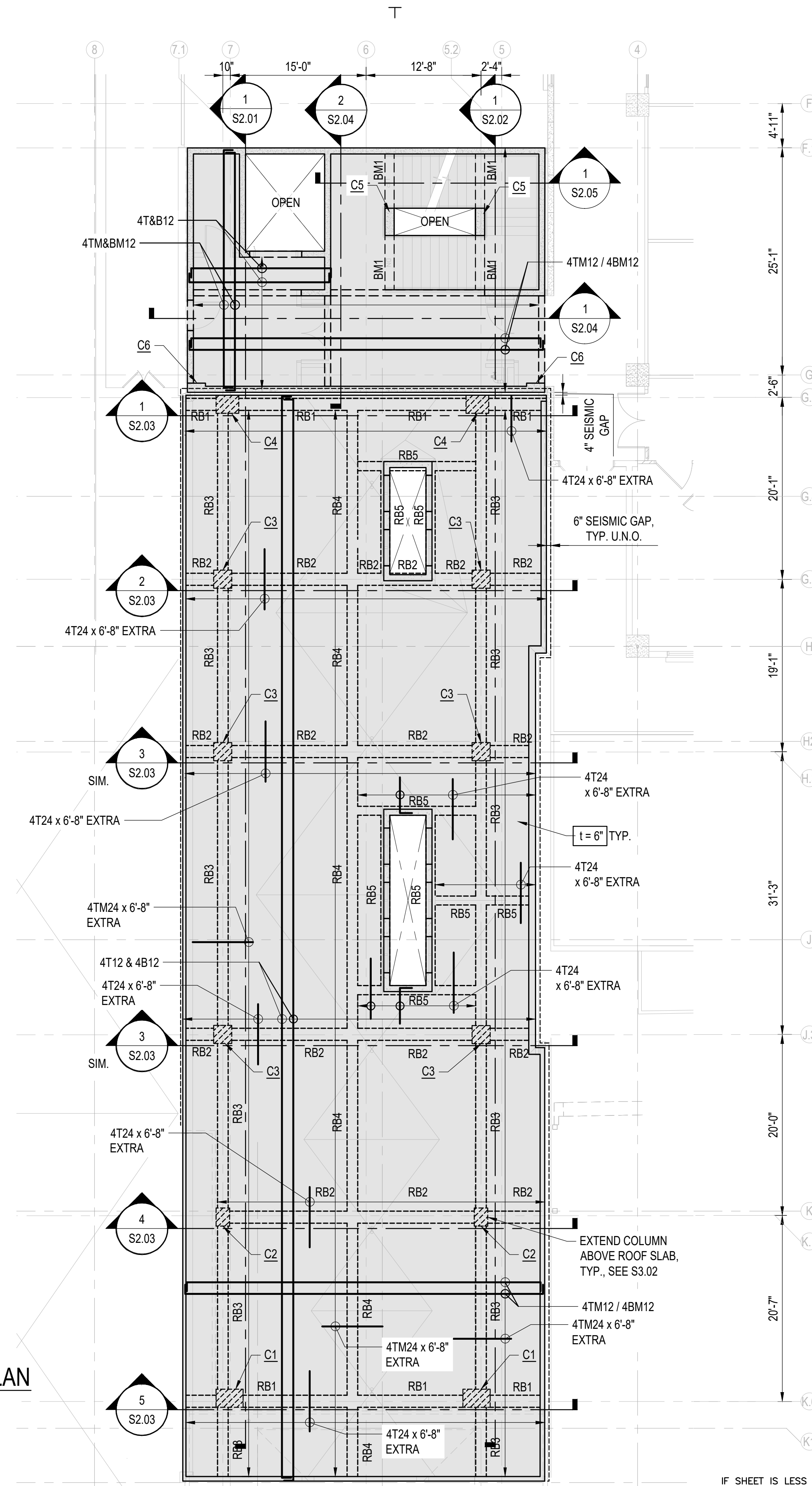
KEYPLAN



SLAB REINF. DETAIL

- LEGEND:
- C1 ----- COLUMN DESIGNATION, SEE SHEET S3.01
 - B1 ----- BEAM DESIGNATION, SEE SHEET S3.01
 - WBC ----- WALL DESIGNATION, SEE SHEET S3.01
 - t=6" ----- SLAB THICKNESS

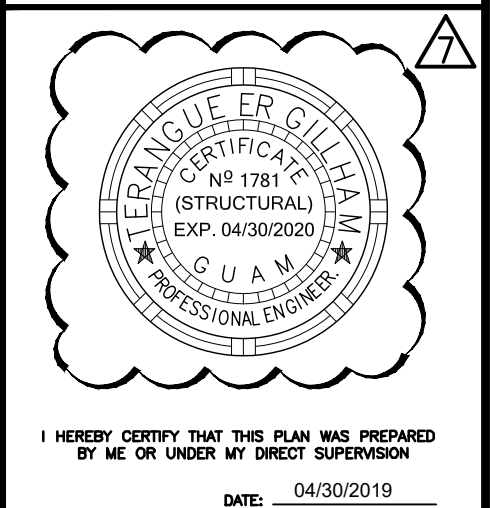
1 3RD LEVEL / ROOF FRAMING PLAN
 S1.03/S1.03 SCALE 1/8" = 1'-0"



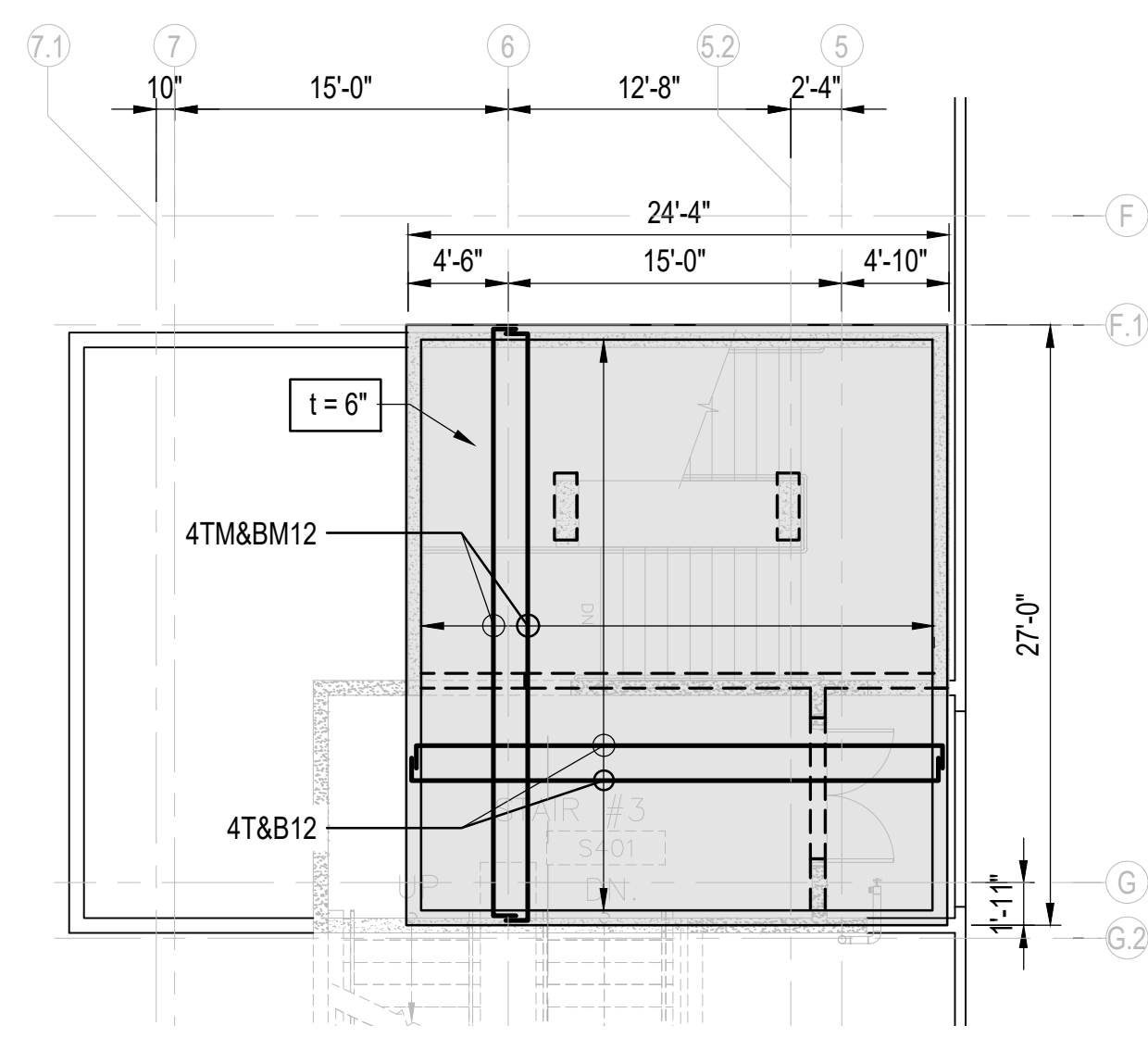
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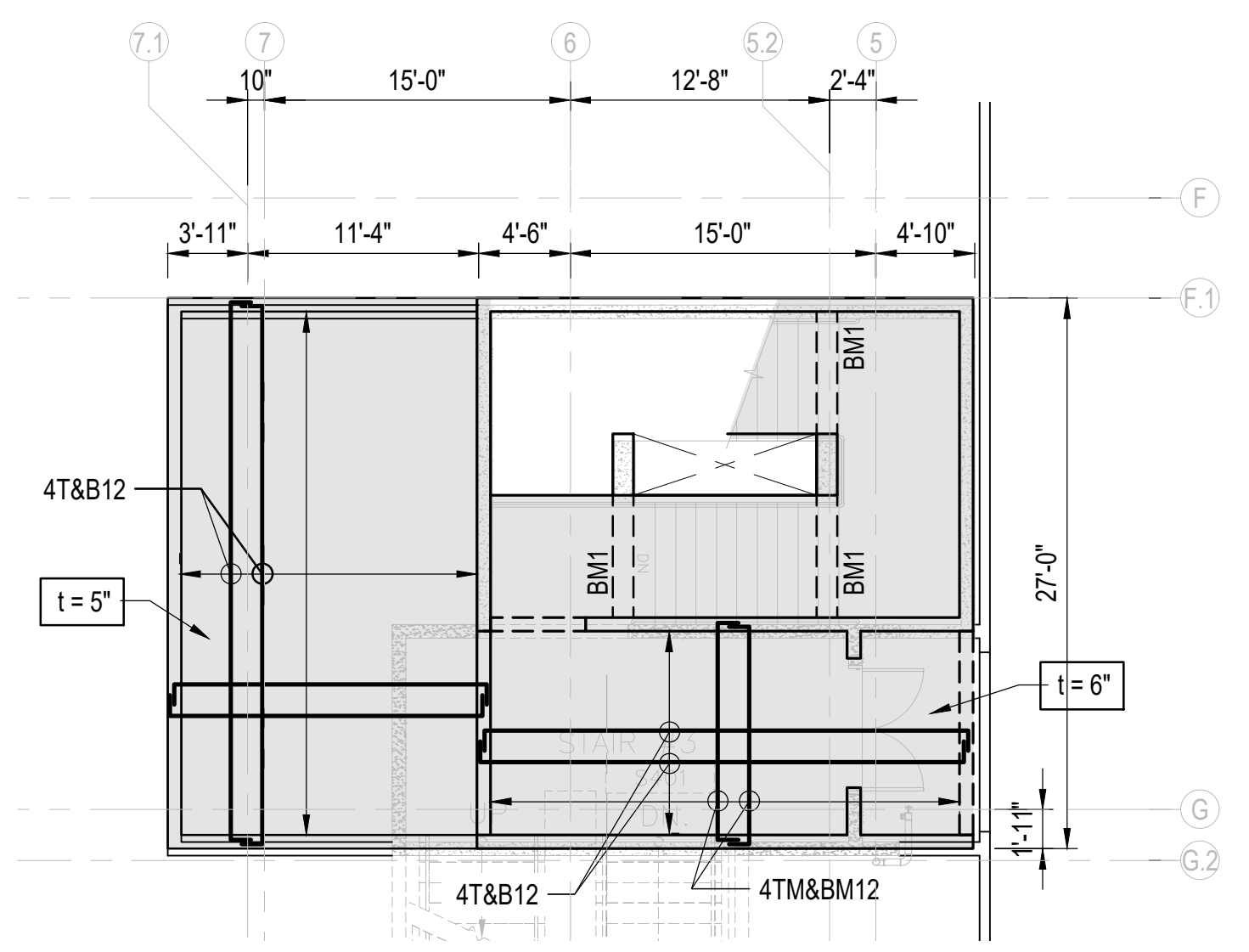
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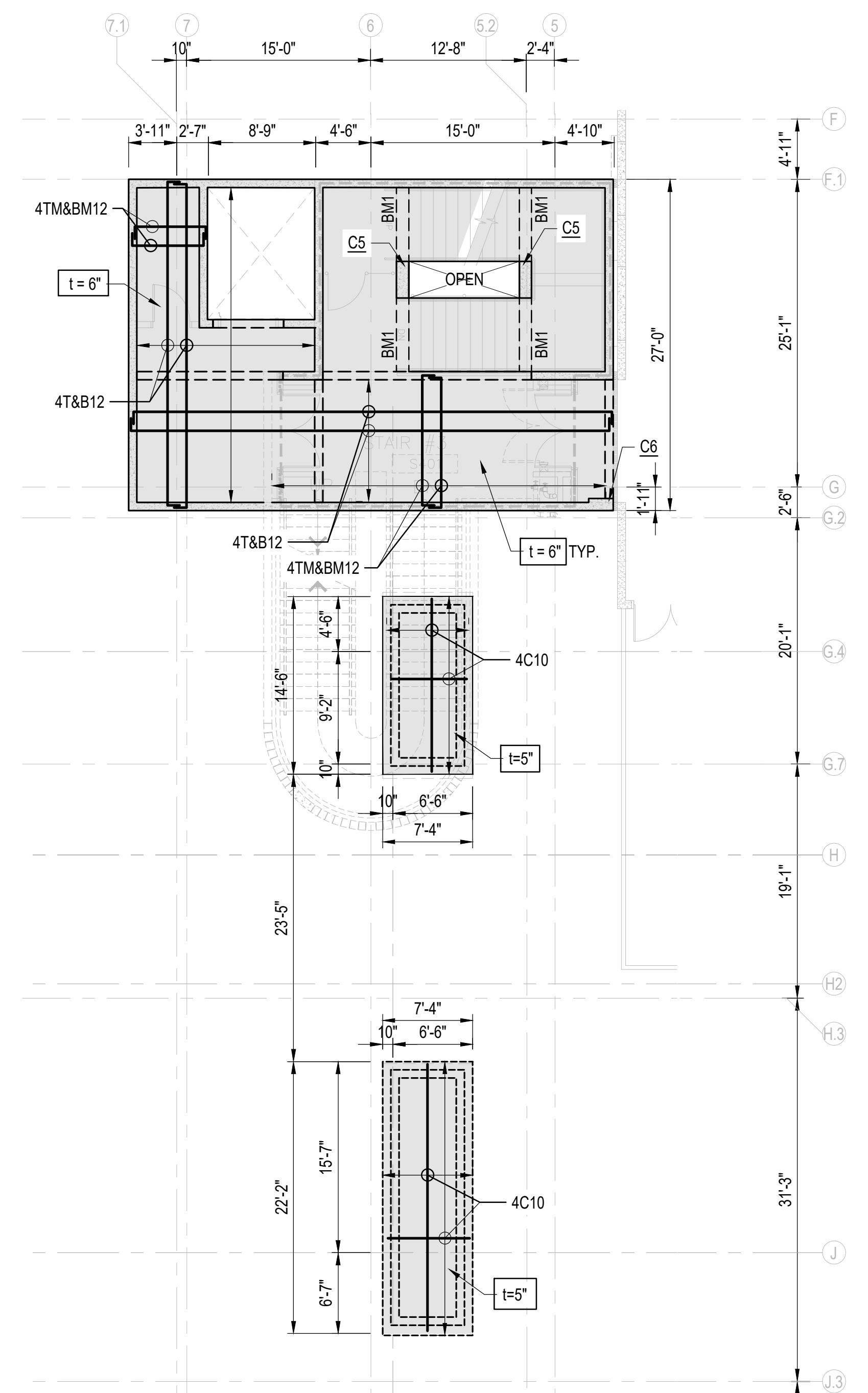
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Designed:	TG
Drawn:	AM
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Supv:	TG
Scale:	AS_SHOWN
Date:	06/06/2017
Project No.:	AutoCAD File
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Sheet No.:	— of —



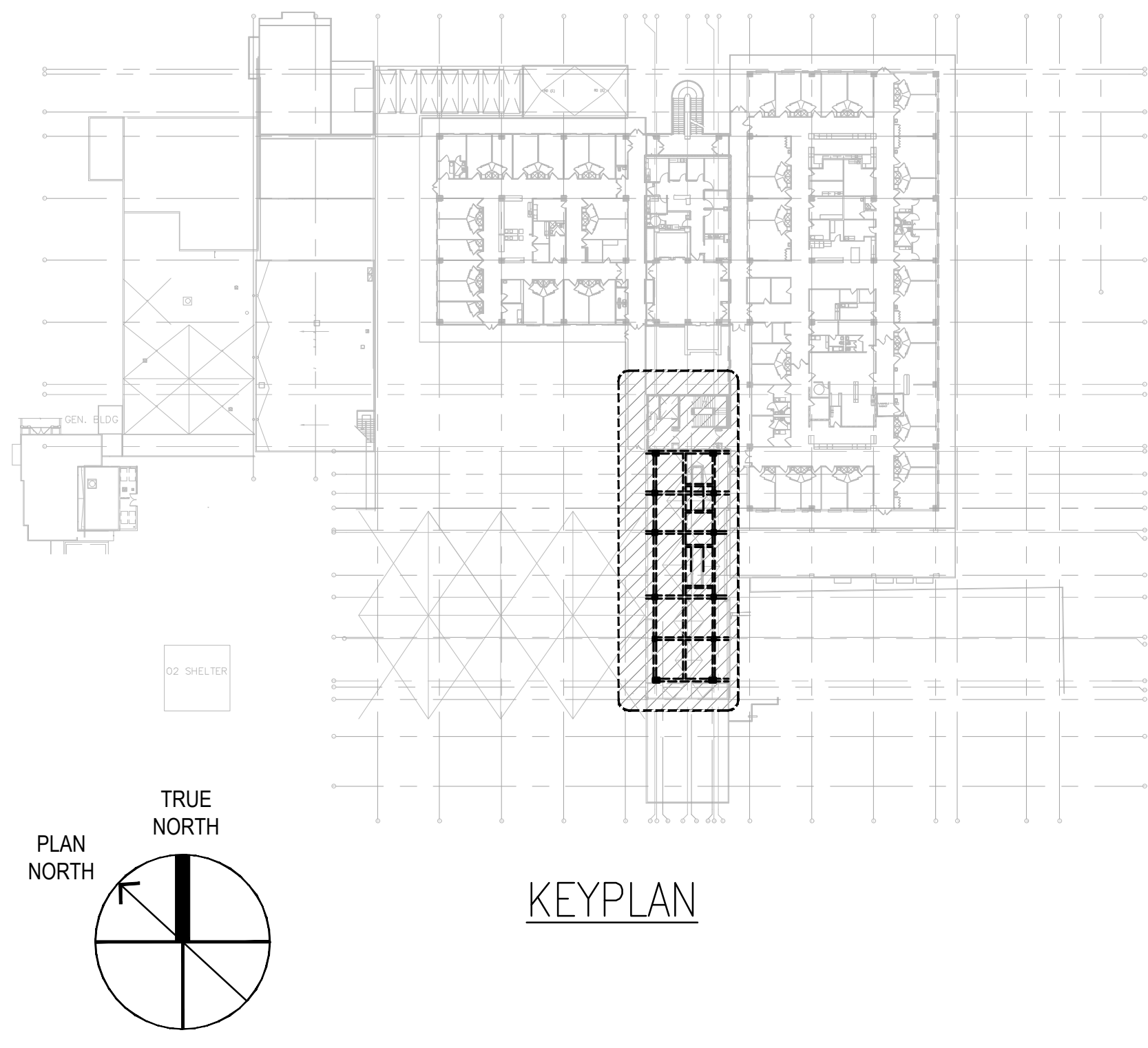
3 UPPER ROOF FRAMING PLAN
 S1.04/S1.04 SCALE 1/8" = 1'-0"



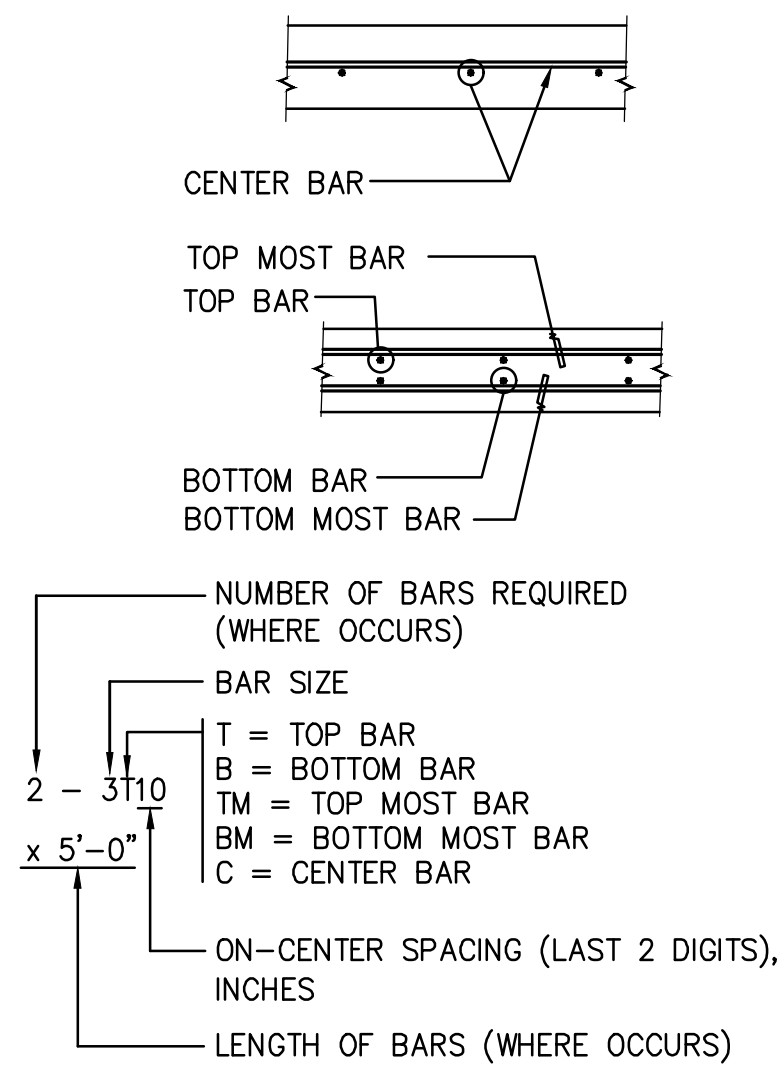
2 LOWER ROOF / STAIR FRAMING PLAN
 S1.04/S1.04 SCALE 1/8" = 1'-0"



1 4TH LEVEL / ROOF FRAMING PLAN
 S1.04/S1.04 SCALE 1/8" = 1'-0"



KEYPLAN



SLAB REINF. DETAIL

- LEGEND:**
- C1 ----- COLUMN DESIGNATION, SEE SHEET S3.01
 - B1 ----- BEAM DESIGNATION, SEE SHEET S3.01
 - WBC ----- WALL DESIGNATION, SEE SHEET S3.01
 - t=6" ----- SLAB THICKNESS

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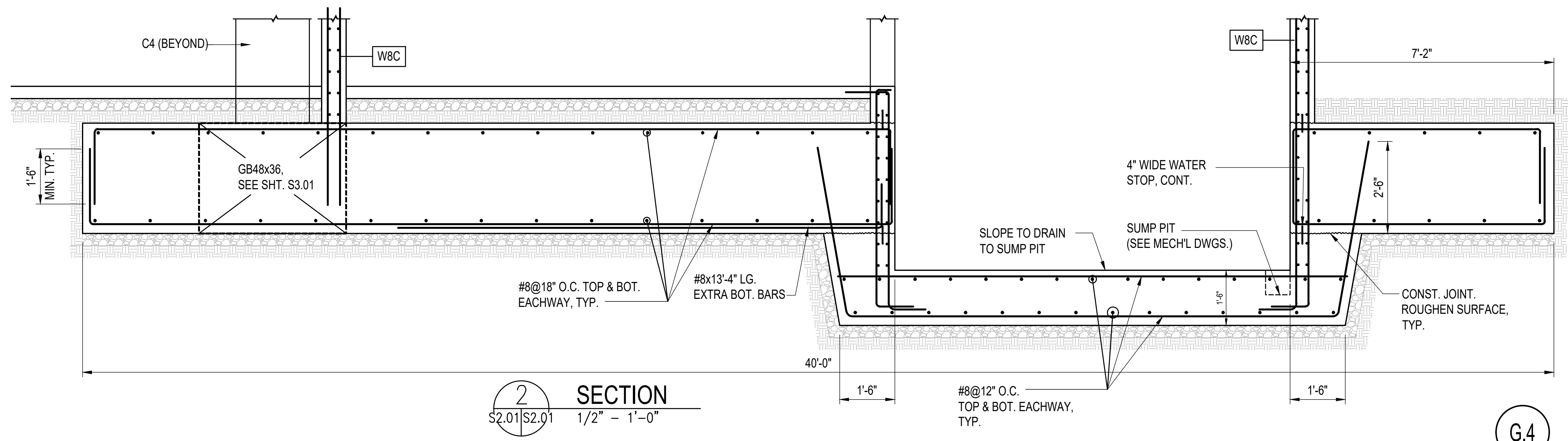
Project: GMH

Title: 4TH LEVEL & ROOF FRAMING PLAN

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 Drawn: AM
 Checked: TG
 Supv: TG
 Scale: AS_SHOWN
 Date: 06/06/2017
 Project No. AutoCAD File
 Drawing No.

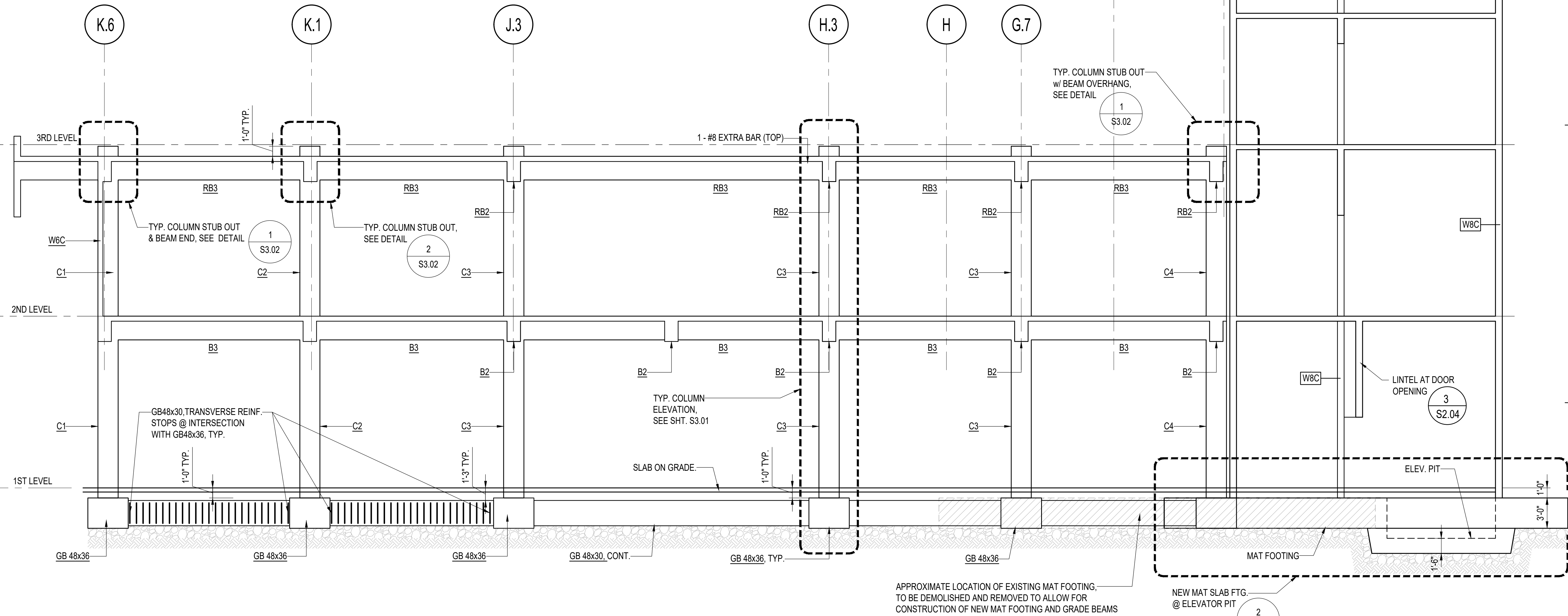
S1.04
 Sheet No. ___ of ___

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SECTION 2
S2.01/S2.01 1/2" - 1'-0"

NOTE:
ALL WALLS W8C UNLESS NOTED OTHERWISE.



SECTION & FRAME ELEVATION 1
S2.01/S2.01 3/16" - 1'-0"

APPROXIMATE LOCATION OF EXISTING MAT FOOTING,
TO BE DEMOLISHED AND REMOVED TO ALLOW FOR
CONSTRUCTION OF NEW MAT FOOTING AND GRADE BEAMS

NEW MAT SLAB FTG.
@ ELEVATOR PIT

REVISIONS		
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Project: GMH

Title: SECTION & FRAME ELEVATION

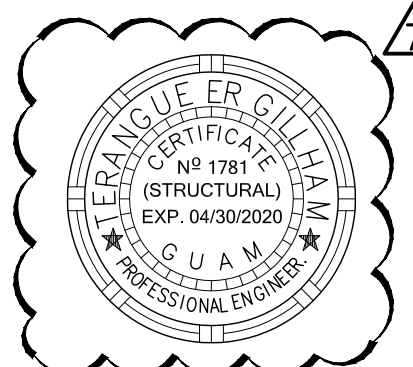
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 Date: 06/06/2017
 Project No. AutoCAD File
 Drawing No.

S2.01
 Sheet No. of

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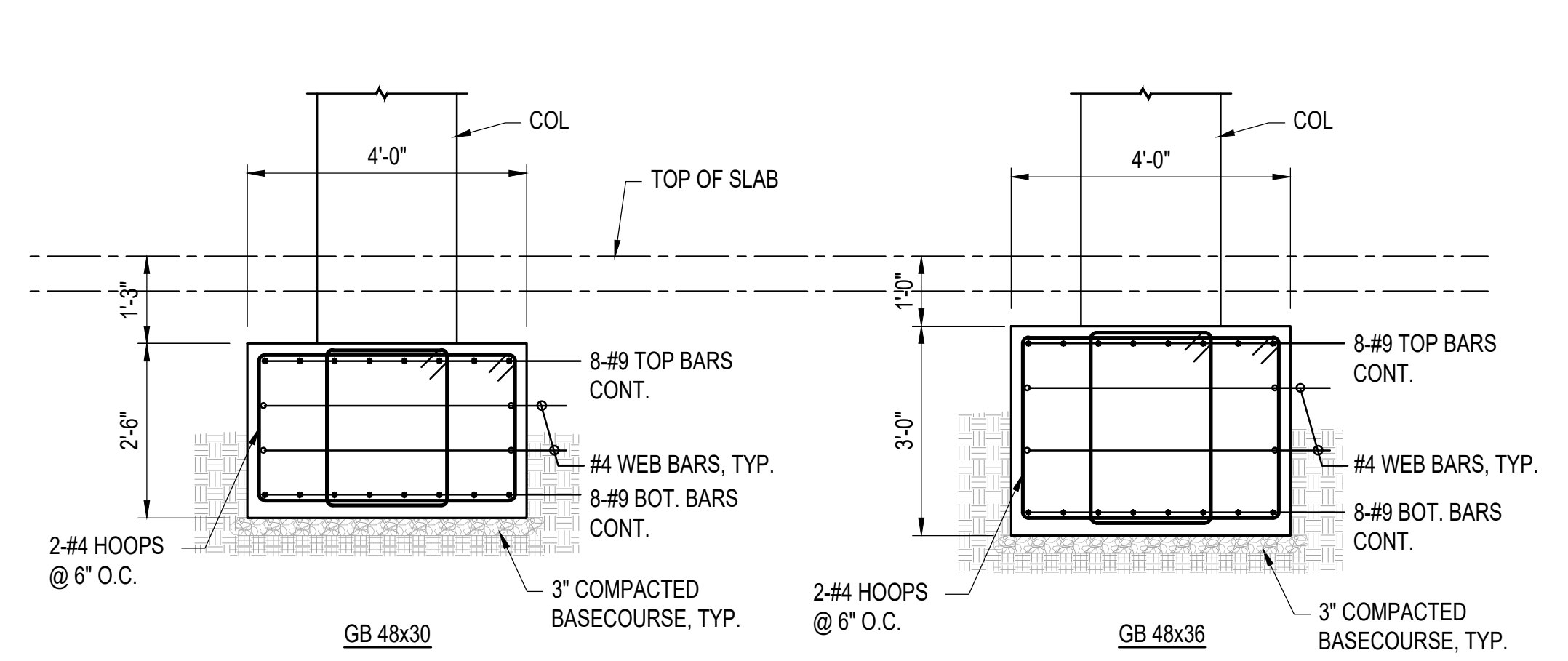
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 DATE: 04/30/2019

Project: GMH

Title: SECTION & FRAME ELEVATION

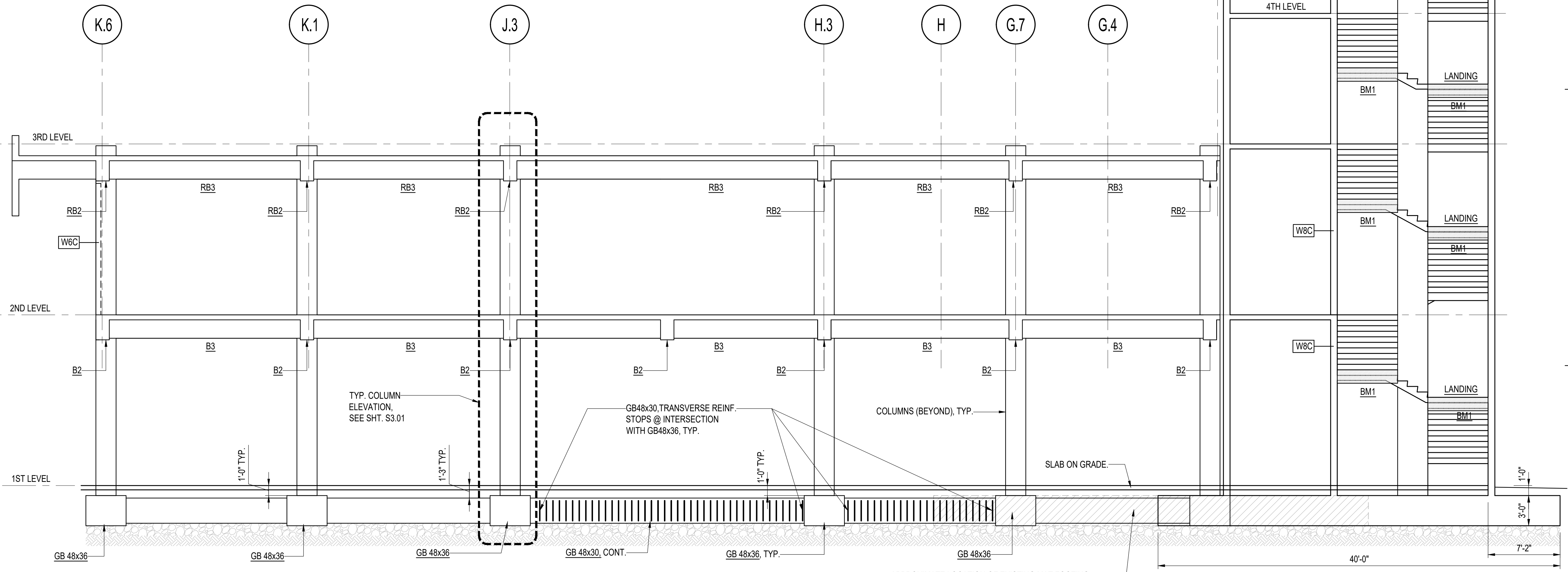
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 Drawn: AM
 Checked: TG
 Supv: TG
 Scale: AS_SHOWN
 Date: 06/06/2017
 Project No. AutoCAD File
 Drawing No.

S2.02
 Sheet No. of



2 GRADEBEAM DETAIL
 S2.02 | S2.02 | 1/2" - 1'-0"

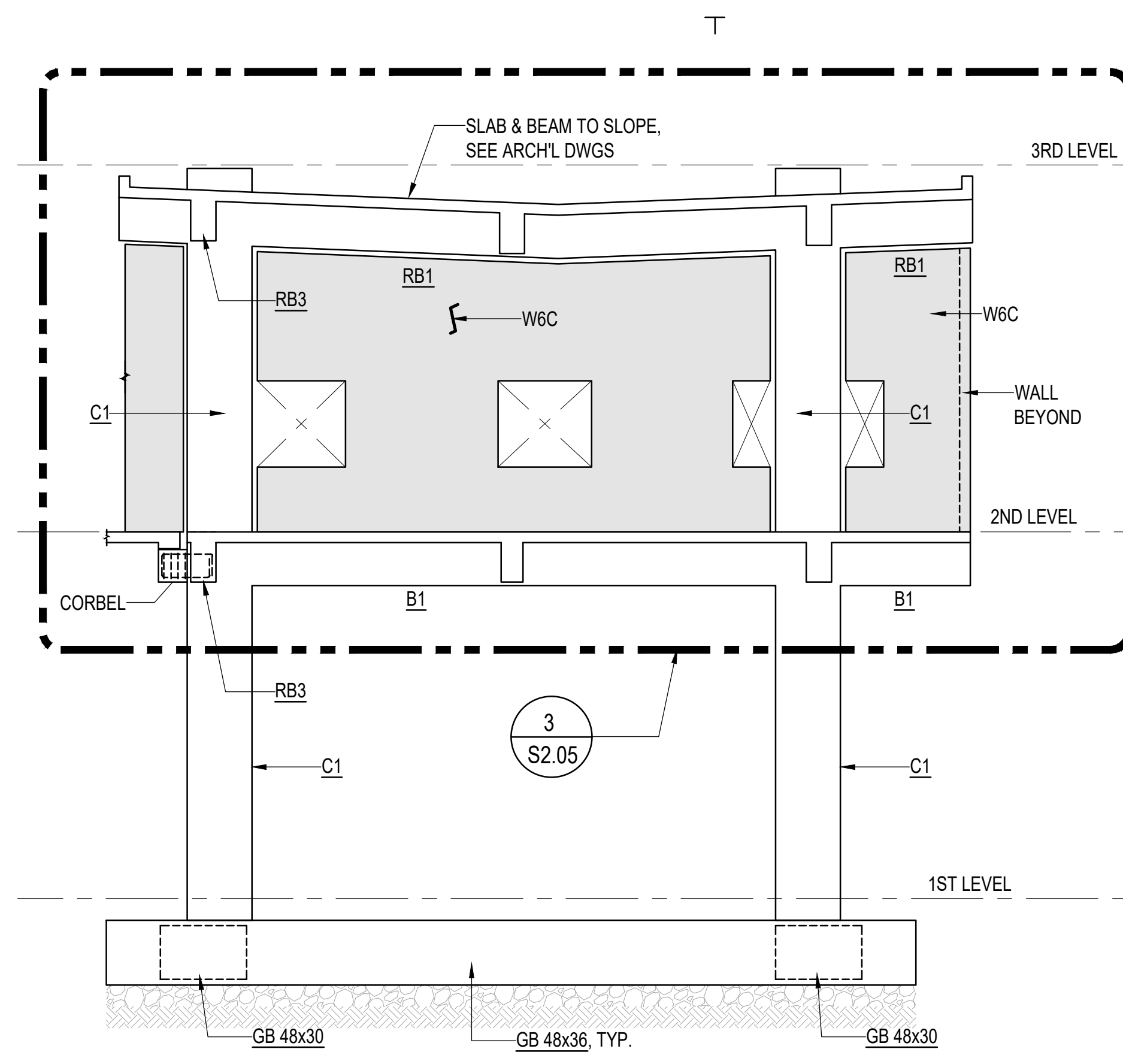
NOTE: ALL WALLS W8C UNLESS NOTED OTHERWISE.



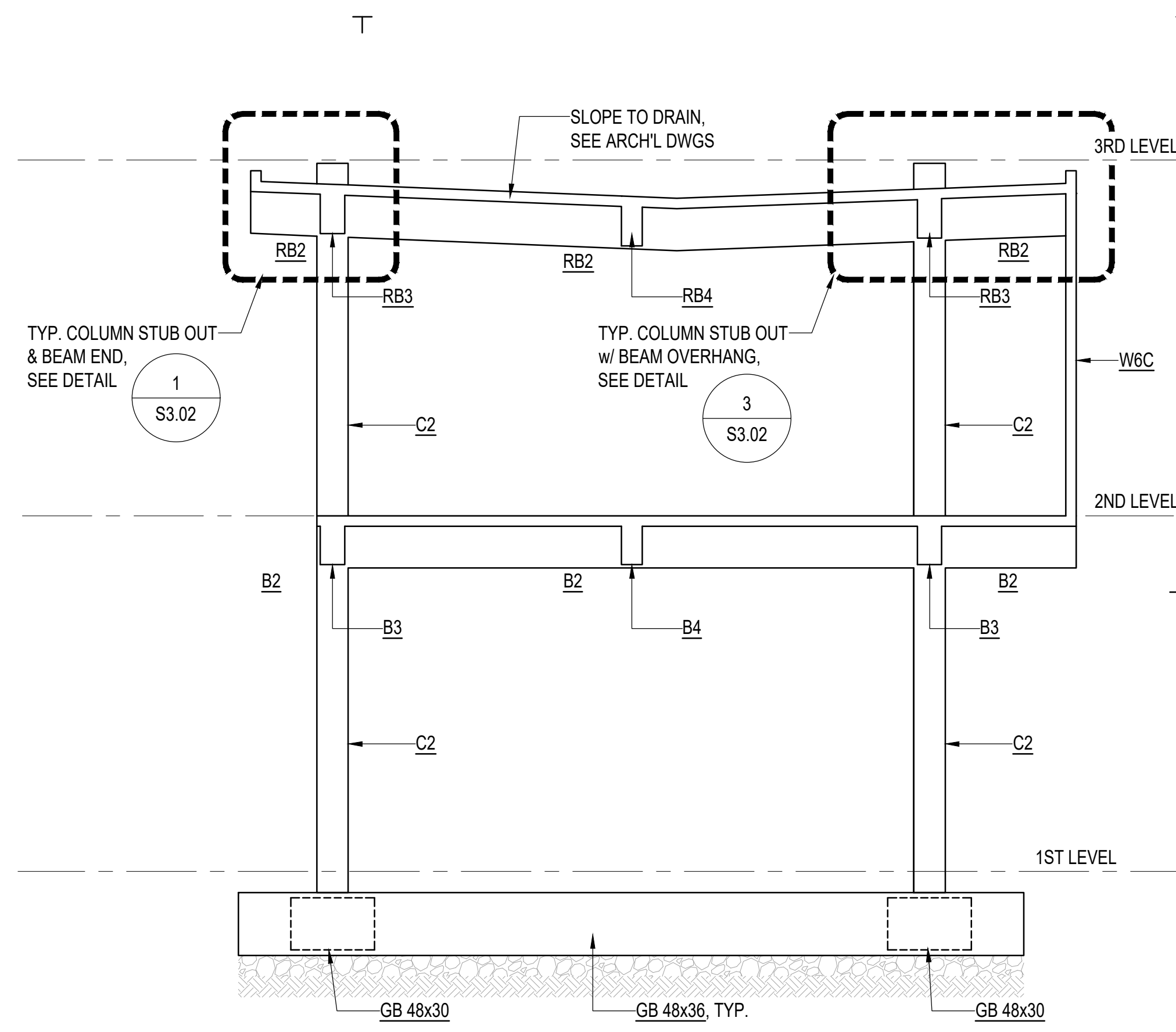
1 SECTION & FRAME ELEVATION
 S2.02 | S2.02 | 3/16" - 1'-0"

APPROXIMATE LOCATION OF EXISTING MAT FOOTING, TO BE DEMOLISHED AND REMOVED TO ALLOW FOR CONSTRUCTION OF NEW MAT FOOTING AND GRADE BEAMS

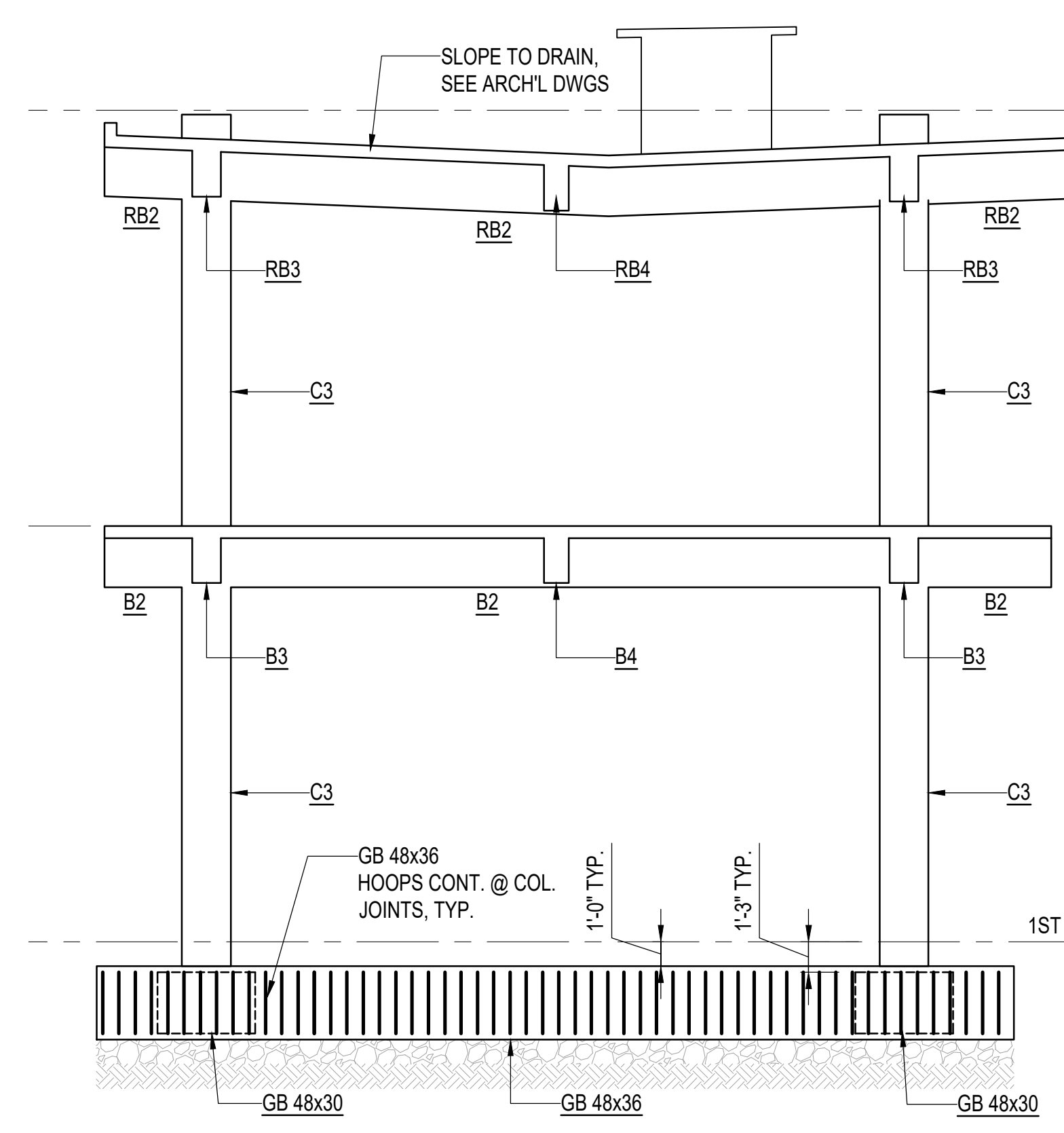
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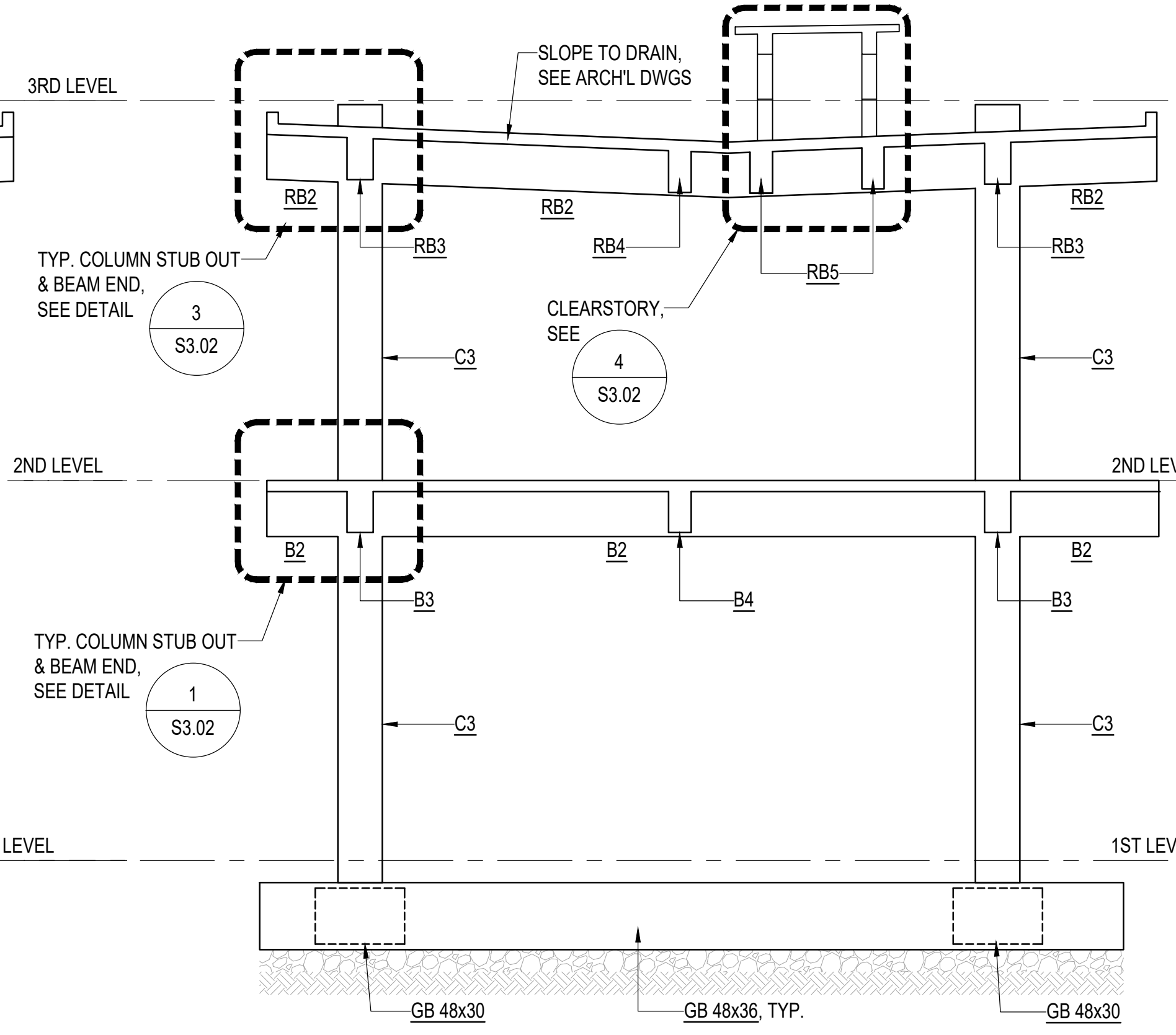
5 ELEVATION
S2.03 | S2.03 3/16" - 1'-0"



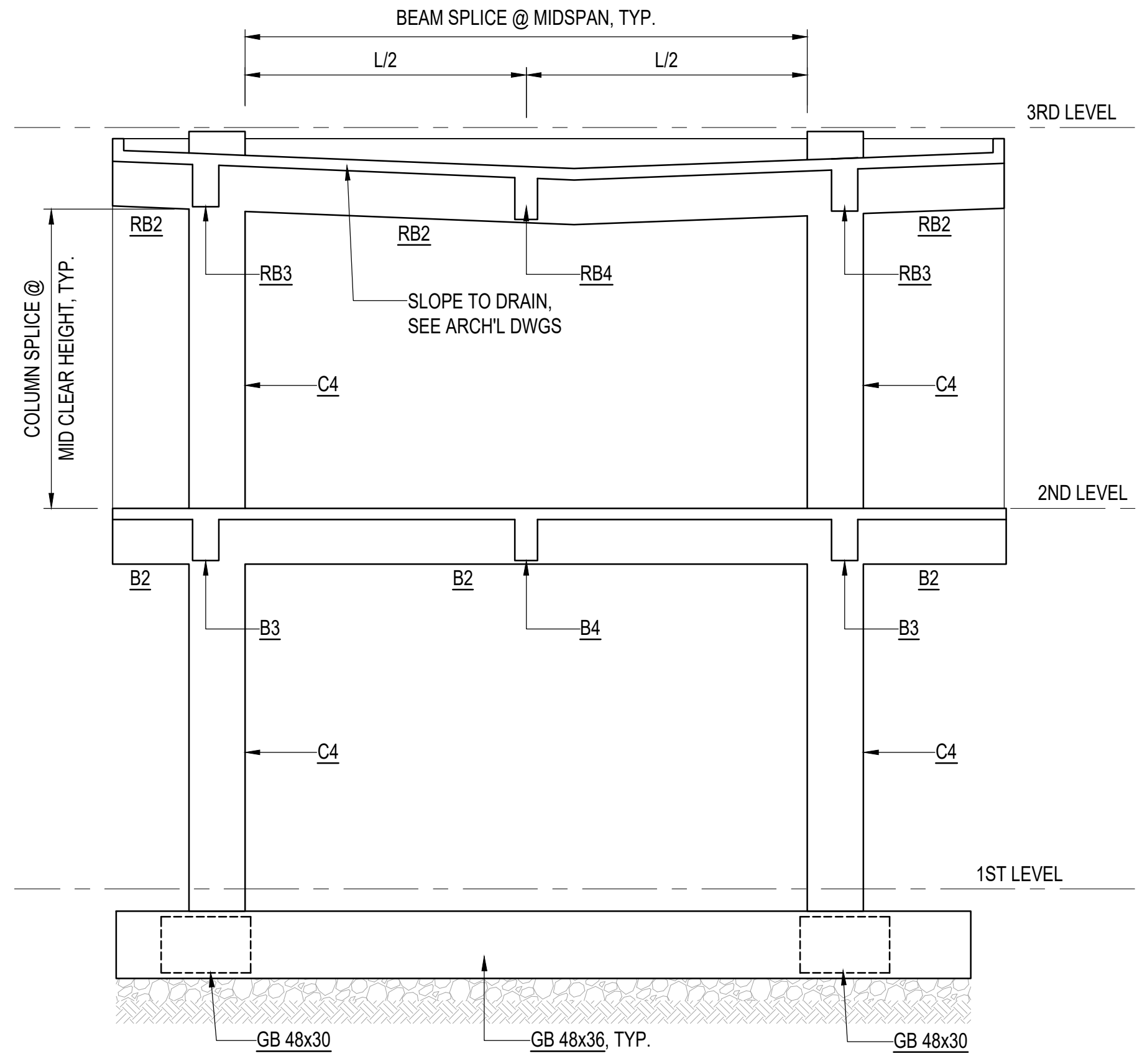
4 ELEVATION
S2.03 | S2.03 3/16" - 1'-0"



3 ELEVATION
S2.03 | S2.03 3/16" - 1'-0"



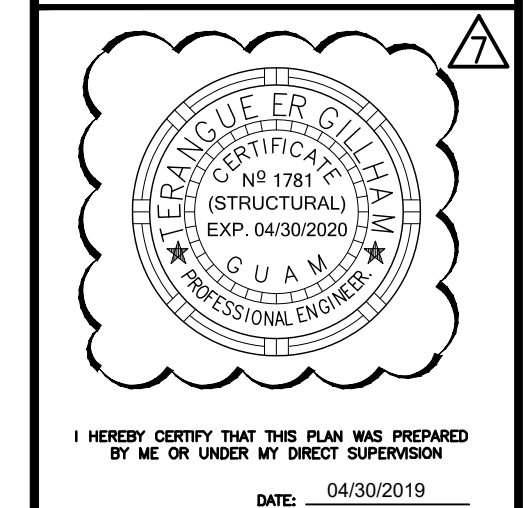
2 ELEVATION
S2.03 | S2.03 3/16" - 1'-0"



1 ELEVATION
S2.03 | S2.03 3/16" - 1'-0"

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1	STAMP UPDATE	04/30/2019

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 Fax: 477-3456
 e-mail: gk2@k2pacific.com



Project: GMH

Title: ELEVATIONS

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 Scale: AS_SHOWN
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 Project No. AutoCAD File
 Drawing No.

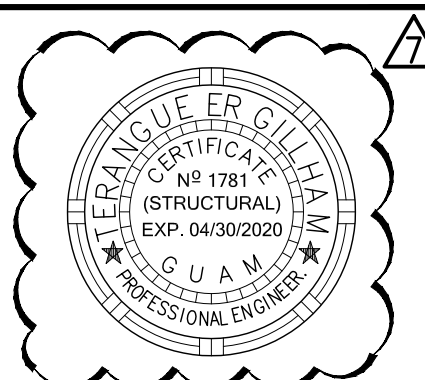
S2.03
 Sheet No. of

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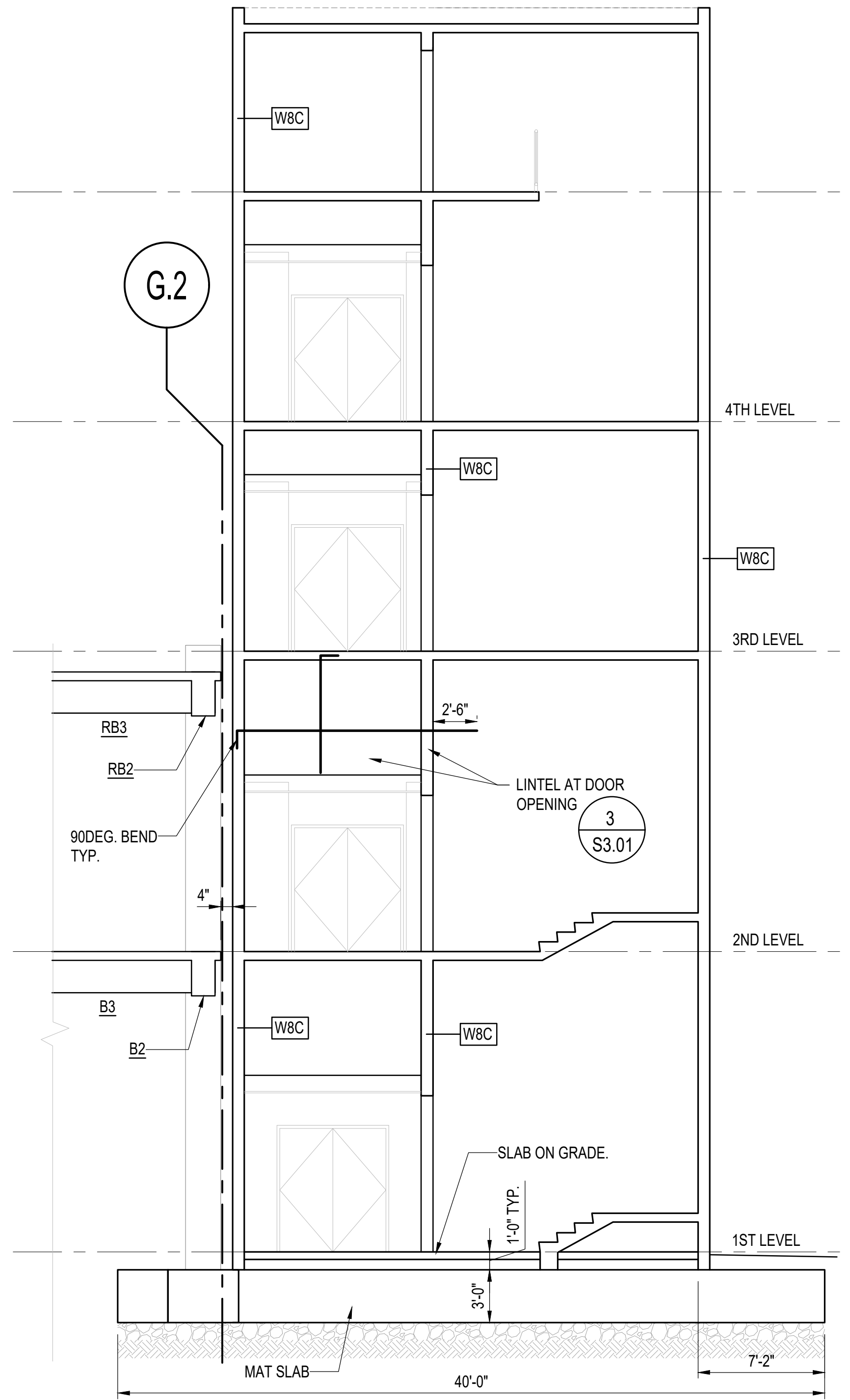
Project: GMH

Title: SECTIONS

Designed: TG
 Drawn: AM
 Checked: TG
 Supv: TG
 Scale: AS_SHOWN
 Date: 06/06/2017
 Project No. AutoCAD File
 Drawing No.

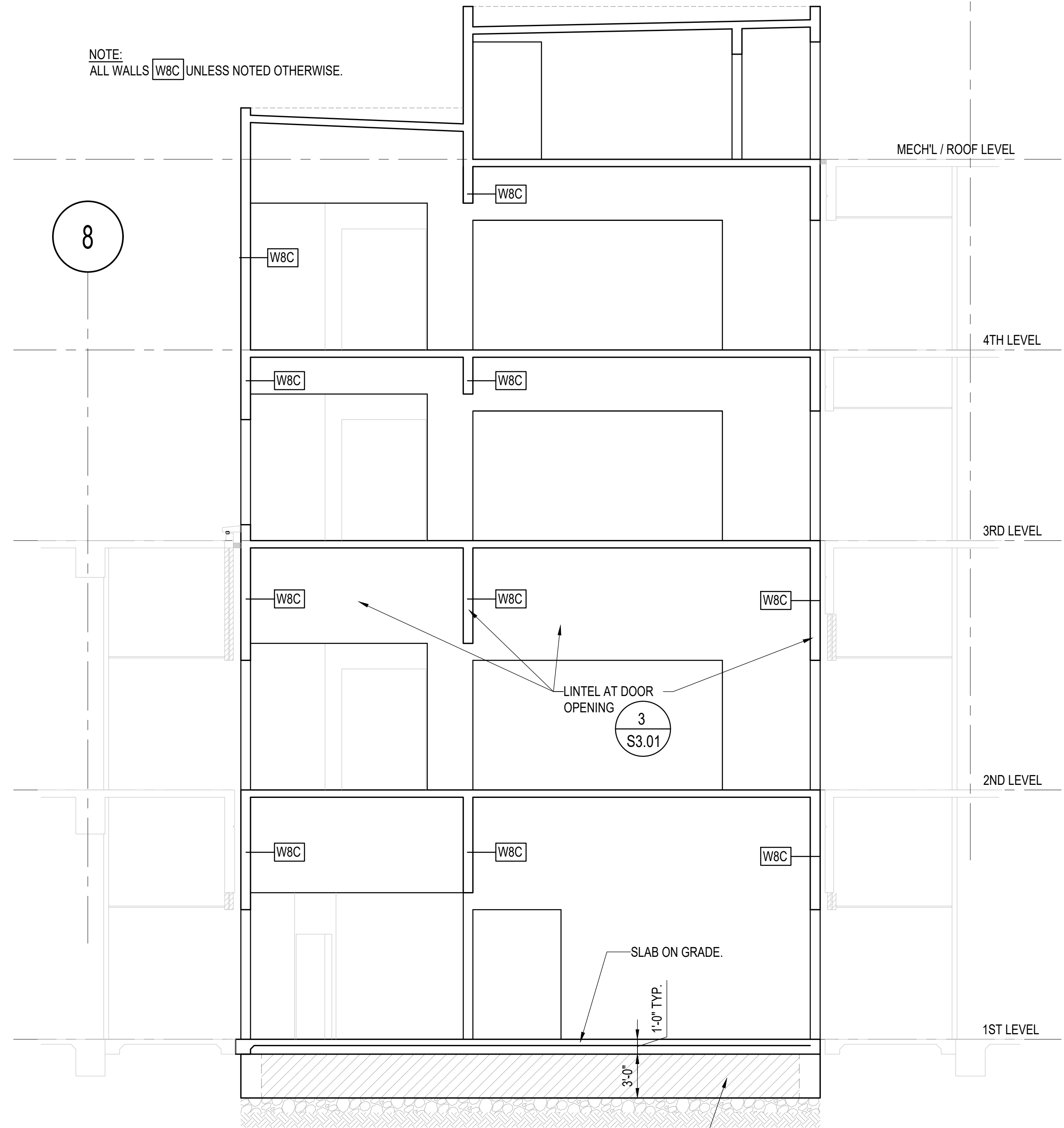
S2.04
 Sheet No. of

NOTE:
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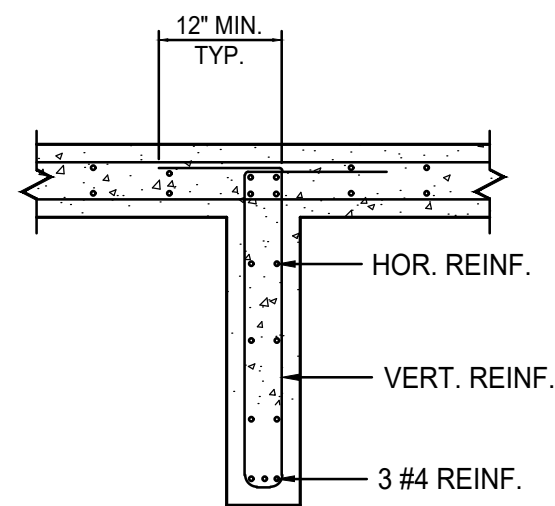


2 SECTION ALONG GL - 6 & 7
 S2.04/S2.04 3/16" - 1'-0"

NOTE:
 ALL WALLS W8C UNLESS NOTED OTHERWISE.



1 SECTION ALONG GL - F & G
 S2.04/S2.04 3/16" - 1'-0"

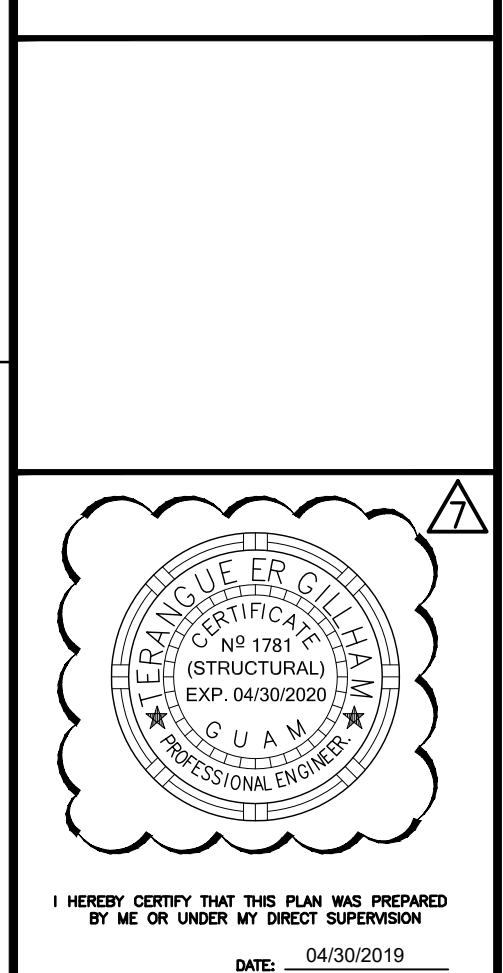


3 TYP. LINTEL AT DOOR OPENING DETAIL
 S2.04/S2.04 NOT TO SCALE

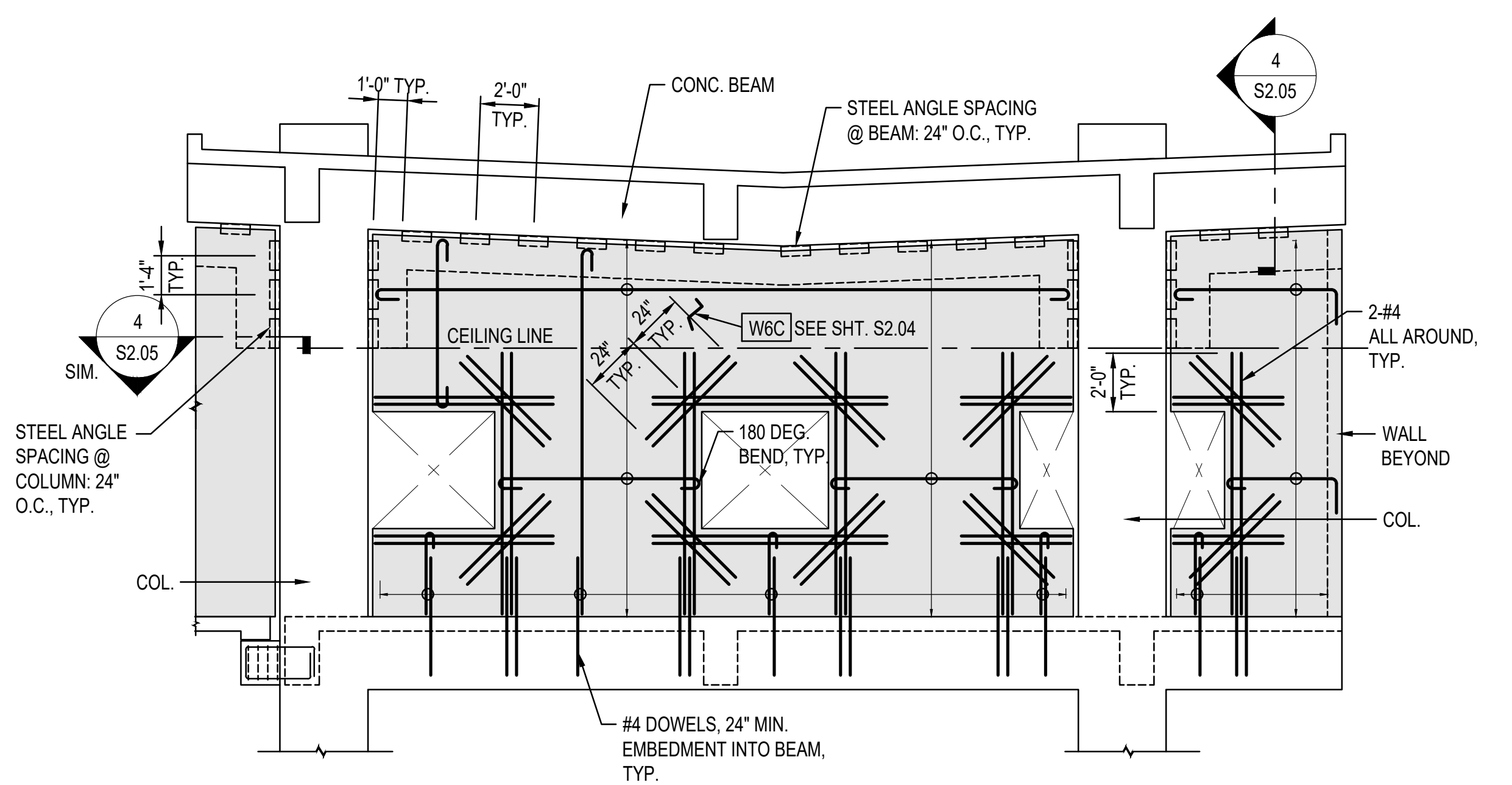
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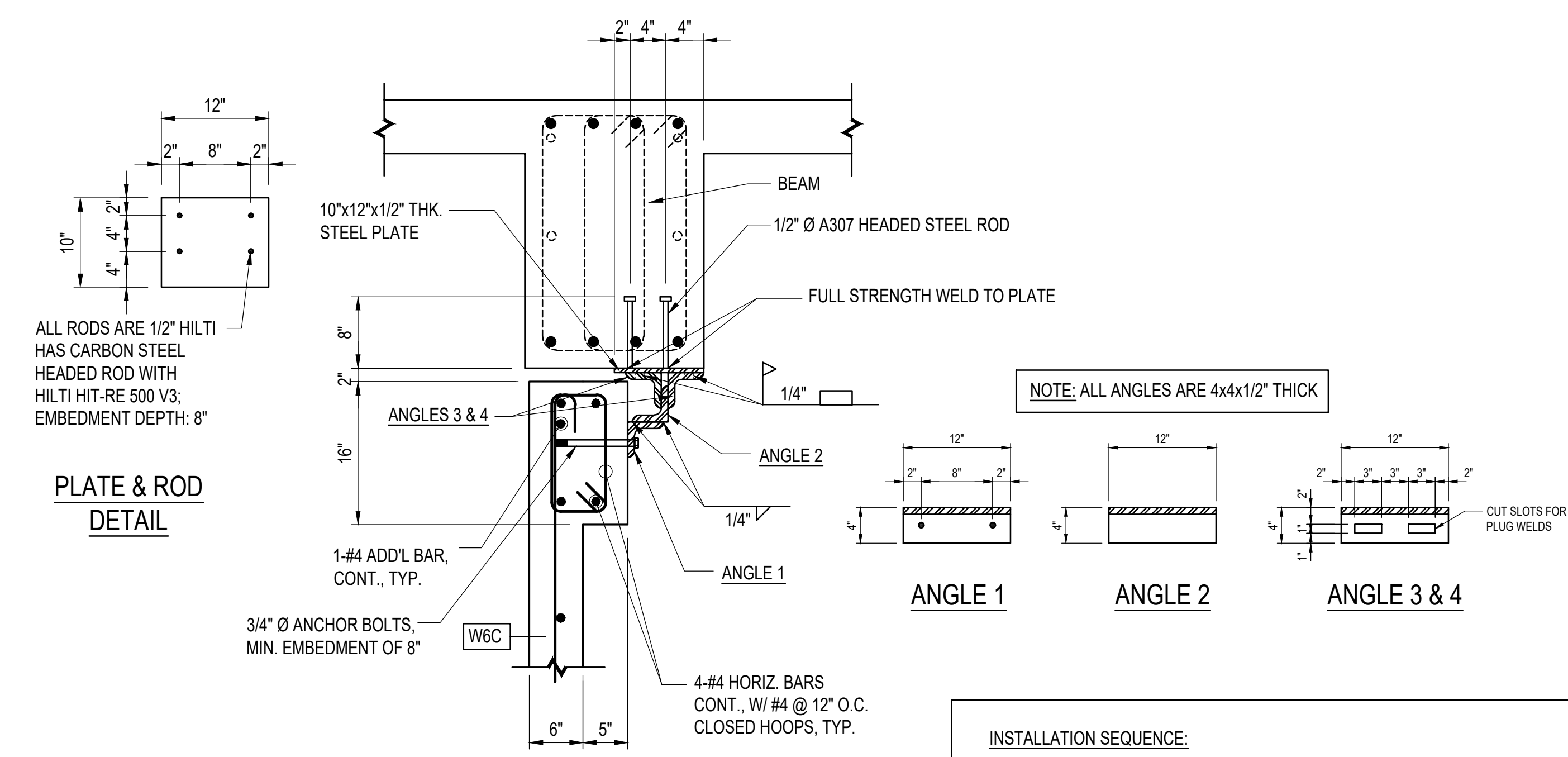
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Scale:	AS_SHOWN
Date:	06/06/2017
Project No.:	AutoCAD File
Drawing No.:	S2.05

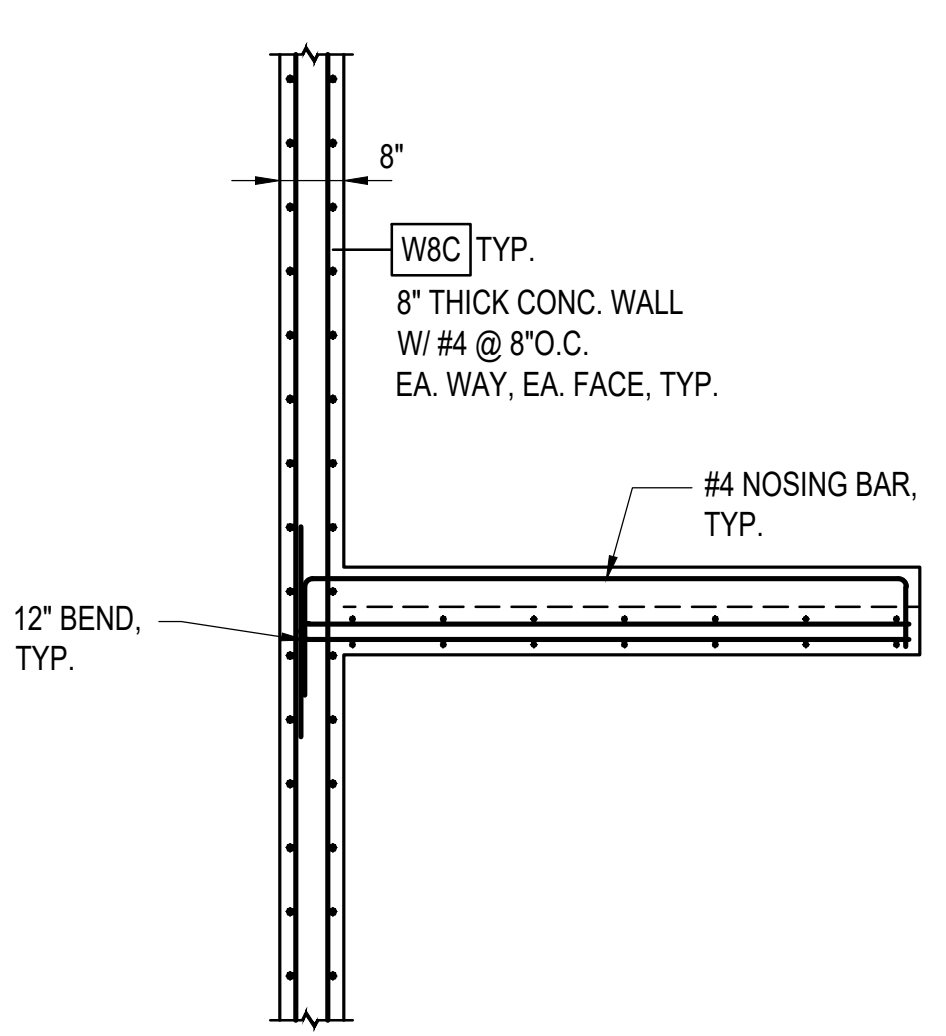


3 WALL ELEVATION
3/16" - 1'-0"
S2.05/S2.05

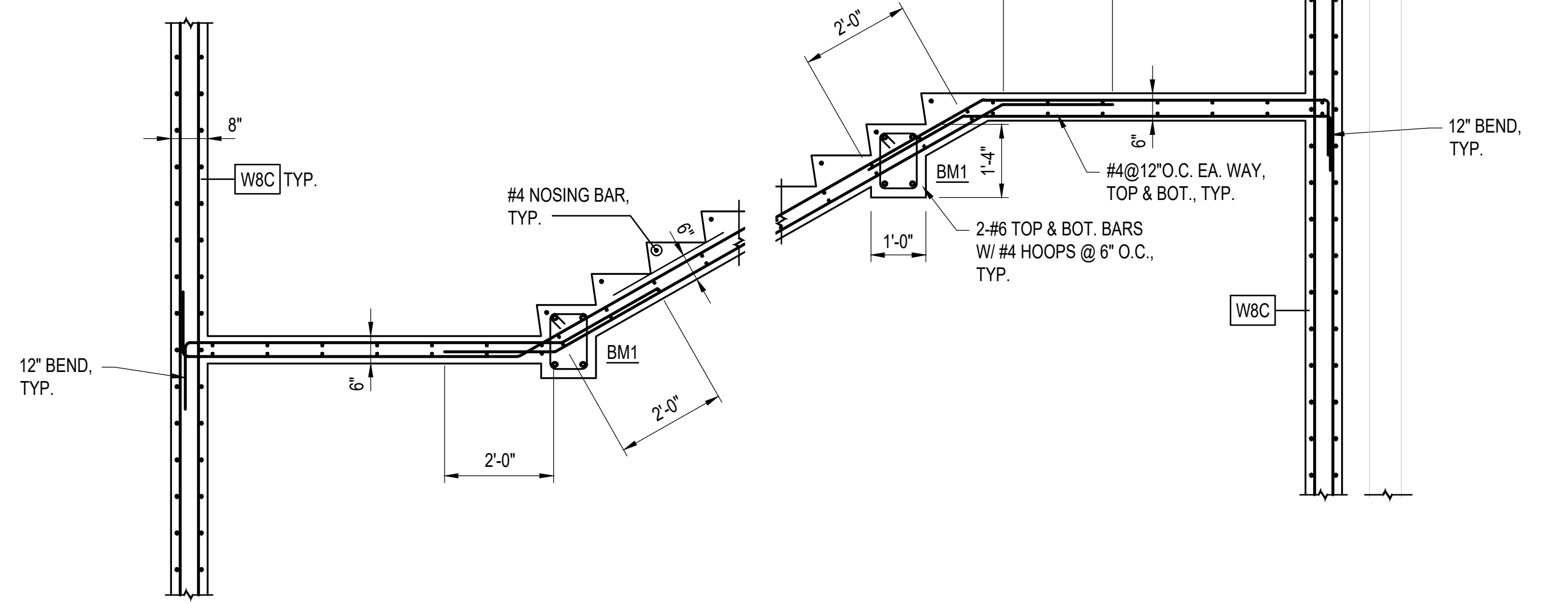


4 CONNECTION DETAIL
1" - 1'-0"
S2.05/S2.05

- INSTALLATION SEQUENCE:
1. CAST HORIZONTAL ANCHOR INTO WALL.
 2. CAST PLATE WITH ANCHOR RODS INTO BEAM ABOVE WALL.
 3. SHOP WELD ANGLES 1 AND 2.
 4. TEMPORARILY INSTALL ANGLE 1/2 ASSEMBLY AND DETERMINE POSITIONS OF ANGLES 3 AND 4 SO THEY WILL STRADDLE ANGLE 2.
 5. REMOVE ANGLE 1/2 ASSEMBLY.
 6. FIELD WELD ANGLES 3 AND 4 INTO PREVIOUSLY DETERMINED POSITIONS.
 7. INSTALL ANGLE 1/2 ASSEMBLY AND SECURE TO HORIZONTAL ANCHOR BOLT.



2 TYP. STAIR NOSING SECTION
1/2" - 1'-0"
S2.05/S2.05

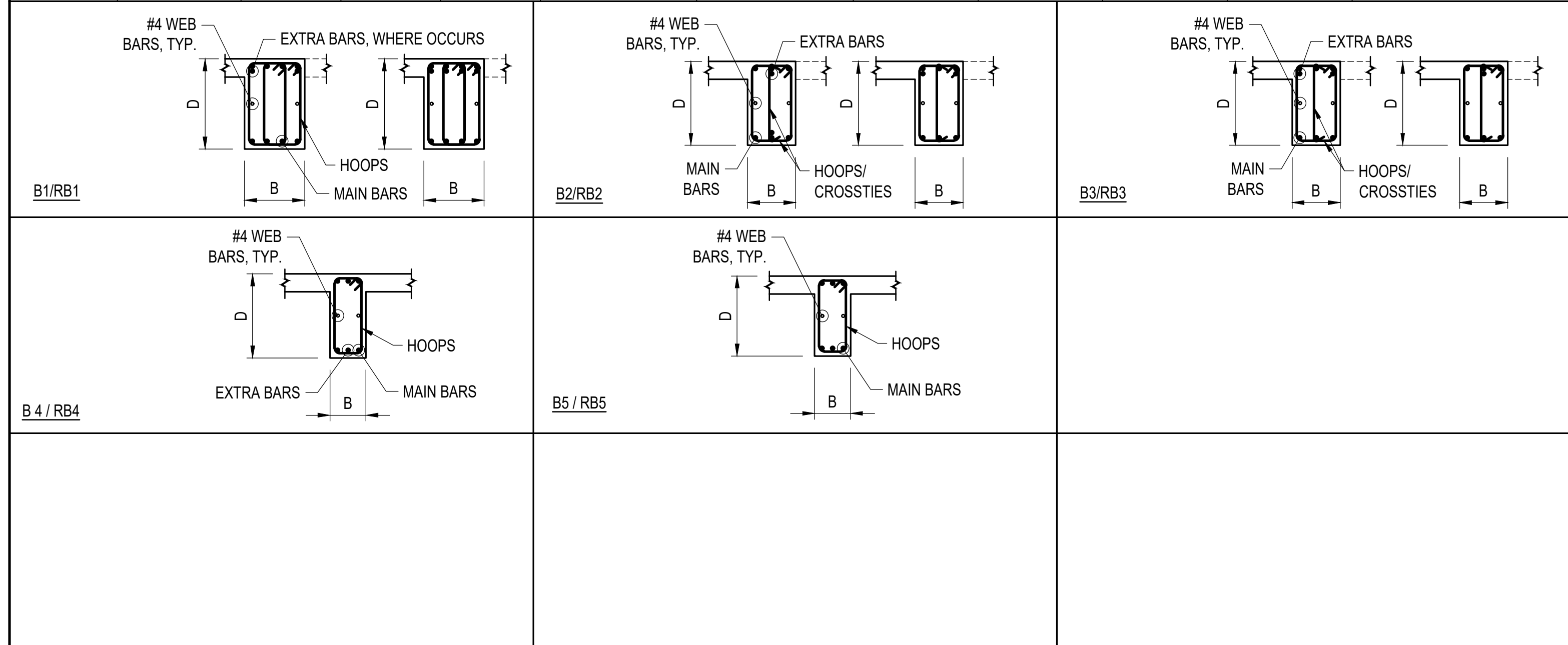


1 TYP. STAIR SECTION
1/2" - 1'-0"
S2.05/S2.05

IF SHEET IS LESS THAN 22" X 34"
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BEAM SCHEDULE

BEAM MARK	SIZE B X D	SECTION	ELEVATION	CONT. TOP BARS	EXTRA TOP BARS		BOTTOM BARS		HOOPS		REMARKS
					LEFT END	RIGHT END	CONT.	EXTRA	H1 (ENDS)	H2 (MID)	
B1 / RB1	20" X 30"	B1 / RB1	S3.2	4 #9	2 #9	3 #9	4 #9	—	#4@4" O.C.	#4@6" O.C.	FRAME ELEVATIONS, SEE SHT. S2.01, S2.02, S2.03.
B2 / RB2	16" X 30"	B2 / RB2	↓	3 #9	1 #9	2 #9	3 #9	—	#4@4" O.C.	#4@6" O.C.	
B3 / RB3	14" X 28"	B3 / RB3	↓	3 #8	2 #8	1 #8	3 #8	—	#4@4" O.C.	#4@6" O.C.	
B4 / RB4	12" X 28"	B4 / RB4	↓	2 #8	1 #8	1 #8	2 #8	1 #8	#4@4" O.C.	#4@6" O.C.	
B5 / RB5	12" X 24"	B5 / RB5	↓	3 #8	—	—	3 #8	—	#4@4" O.C.	#4@6" O.C.	



COLUMN SCHEDULE

COLUMN DESIGNATION	SECTION	REINFORCEMENT		REMARKS
		REBAR DESIGNATION	REINFORCING	
C1		VERTICAL BARS	16 - #10	COLUMN ELEVATION, SHEET S3.01
		CLOSED HOOPS	#4 @ 4" O.C. (4 - PER SET)	
C2		VERTICAL BARS	12 - #10	
		CLOSED HOOPS	#4 @ 4" O.C. (3 - PER SET)	
C3		VERTICAL BARS	12 - #10	
		CLOSED HOOPS	#4 @ 4" O.C. (3 - PER SET)	
C4		VERTICAL BARS	16 - #10	
		CLOSED HOOPS	#4 @ 4" O.C. (4 - PER SET)	
C5 (@STAIR CASE)		VERTICAL BARS	8 - #8	
		CLOSED HOOPS	#4 @ 6" O.C. (2 - PER SET)	
C6 @ END WALL)		VERTICAL BARS	10 - #8	
		CLOSED HOOPS	#4 @ 6" O.C. (2 - PER SET)	

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WALL SCHEDULE

W6C - 6" THICK, #4@8" O.C. VERTICAL AND HORIZONTAL BARS SINGLE LAYER

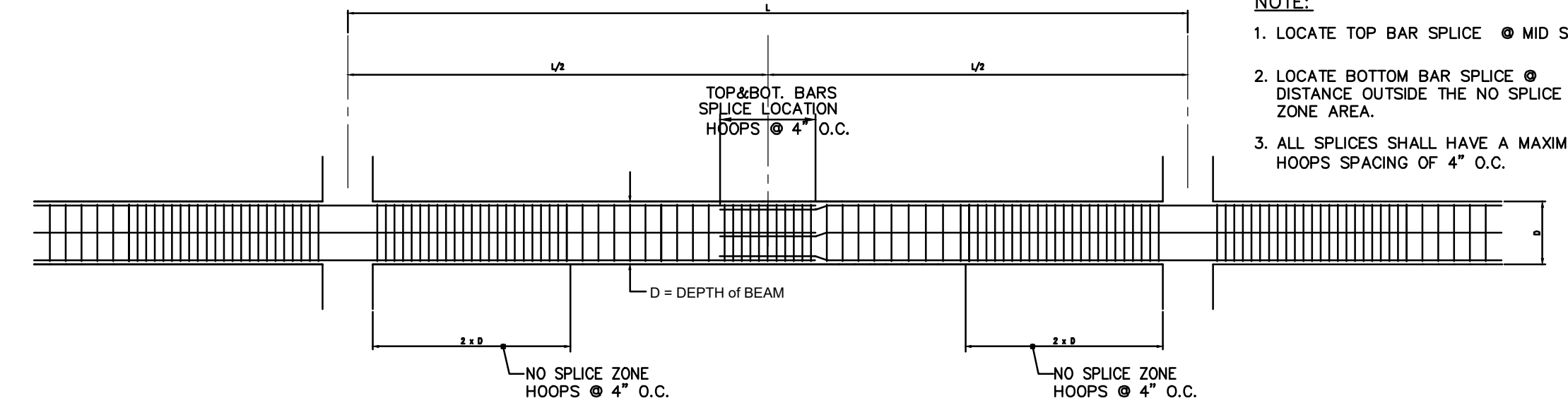
W8C - 8" THICK, #4@8" O.C. VERTICAL AND HORIZONTAL BARS EACH WAY EACH FACE, WITH HORIZONTAL BARS AS OUTER BARS.

W8C LINTELS AT DOOR OPENINGS - 8" THICK, #4@6" HORIZONTAL BARS, #4@6" O.C. VERTICAL BARS AS CLOSED HOOPS WITH 90 DEG. HOOKS AT TOP (SHEET S3.1)

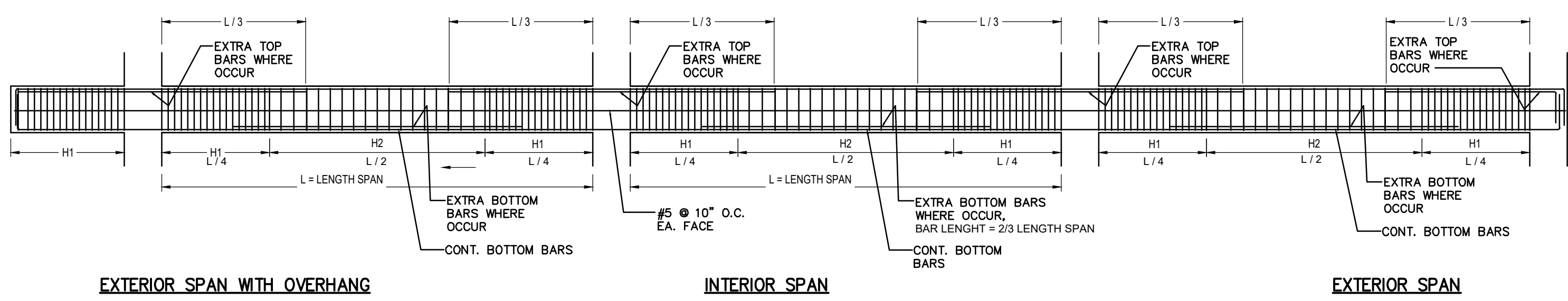
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Project:	GMH
Title:	COLUMN & BEAM SCHEDULE and TYP. BEAM ELEVATIONS
Designed:	TG
Drawn:	AM
Checked:	TG
Supv:	TG
Scale:	AS SHOWN
Date:	06/06/2017
Project No.:	AutoCAD File
Drawing No.:	S3.01

- NOTE:**
1. LOCATE TOP BAR SPLICE @ MID SPAN
 2. LOCATE BOTTOM BAR SPLICE @ DISTANCE OUTSIDE THE NO SPLICE ZONE AREA.
 3. ALL SPLICES SHALL HAVE A MAXIMUM HOOPS SPACING OF 4" O.C.

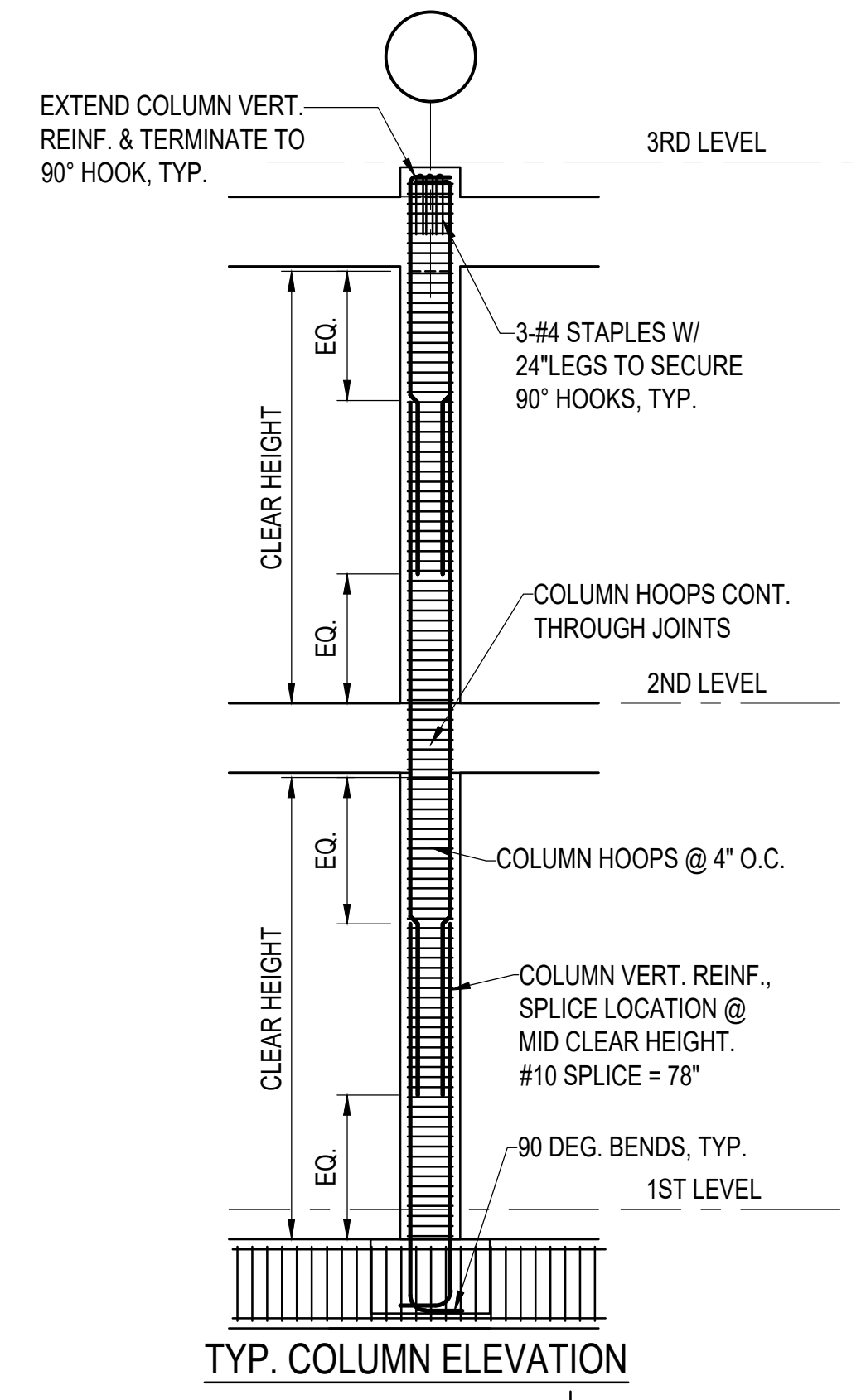


TYPICAL BEAM SPLICE DETAIL

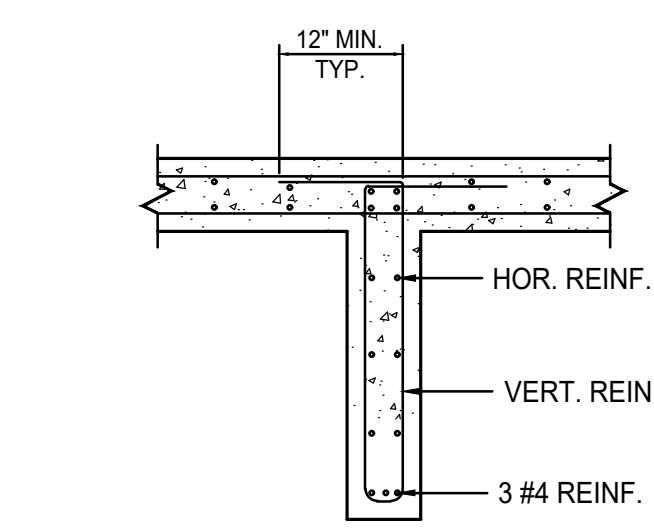


TYPICAL BEAM ELEVATION

CONTRACTOR TO PROVIDE SHOP DRAWINGS PRIOR TO REBARS FABRICATION FOR REVIEW AND APPROVAL.

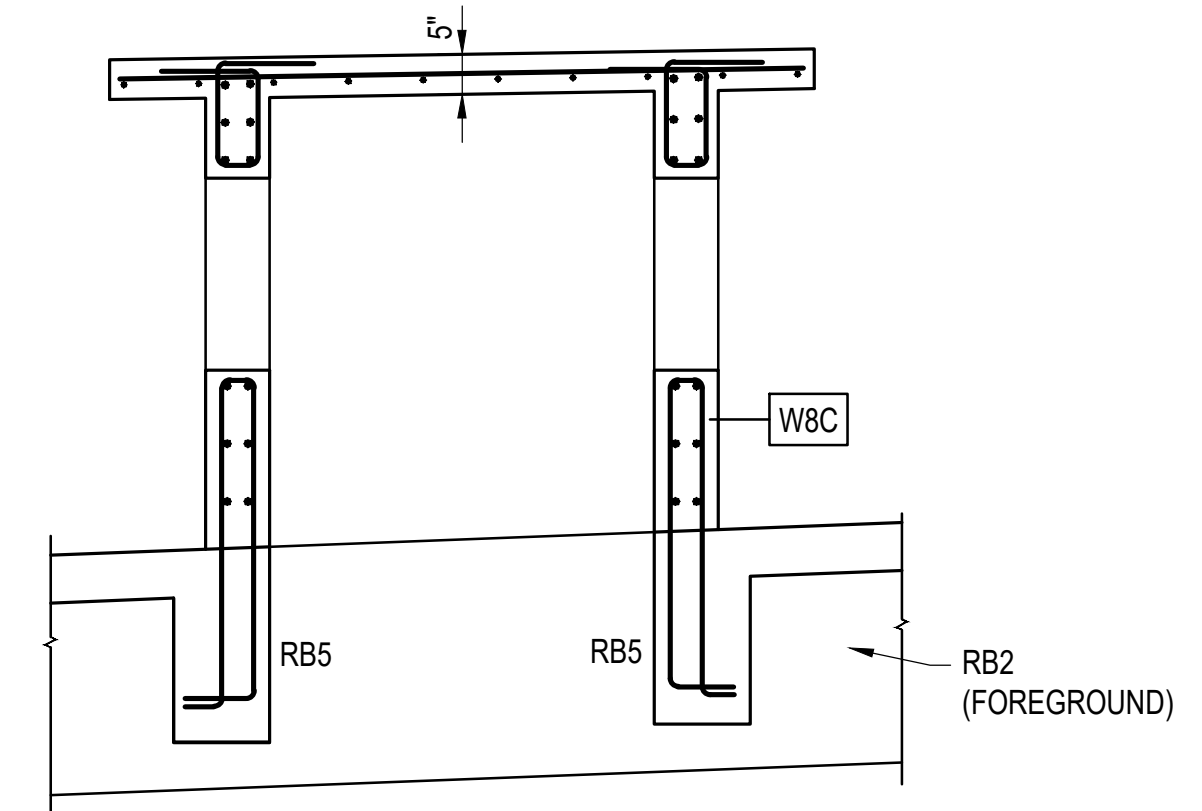
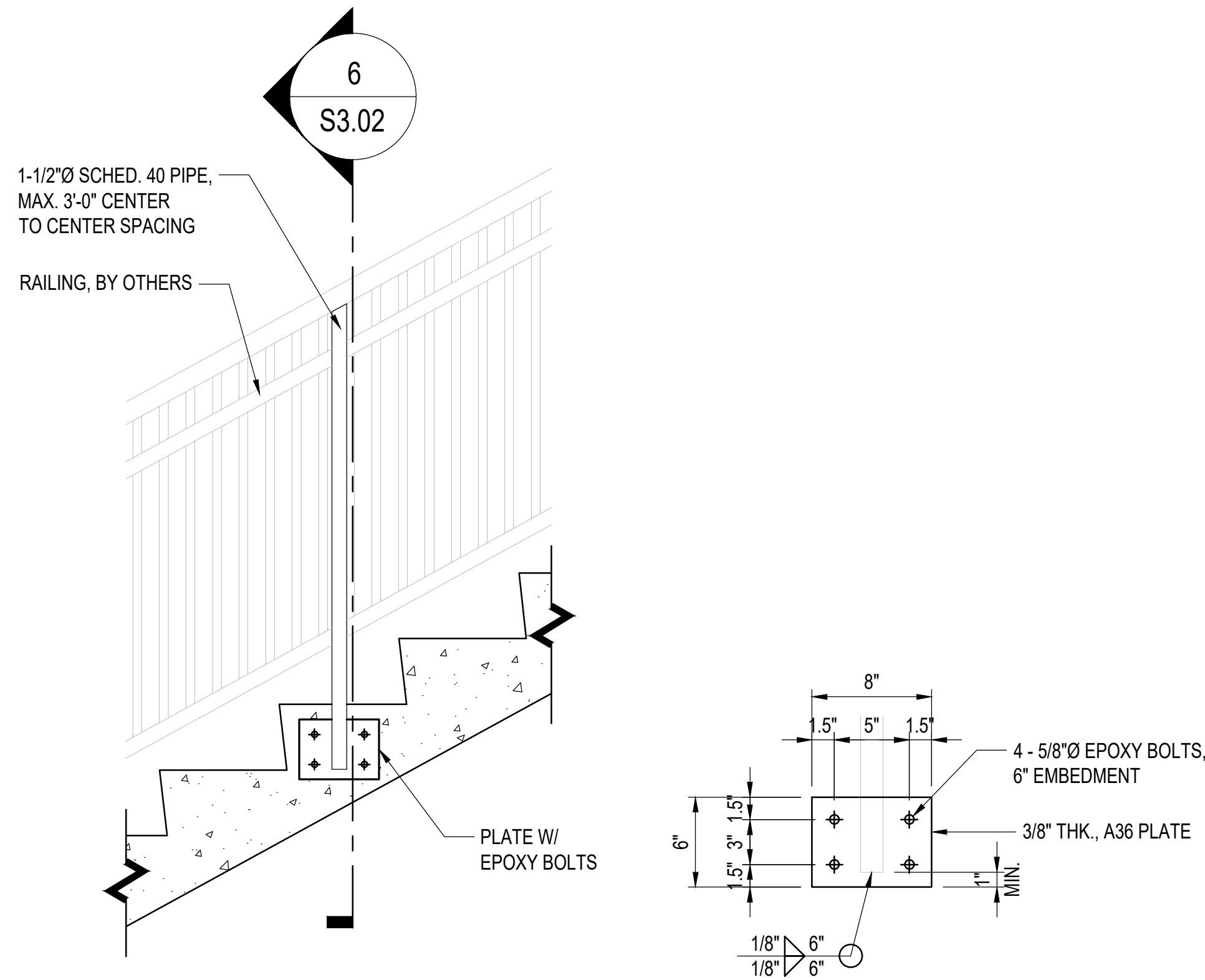
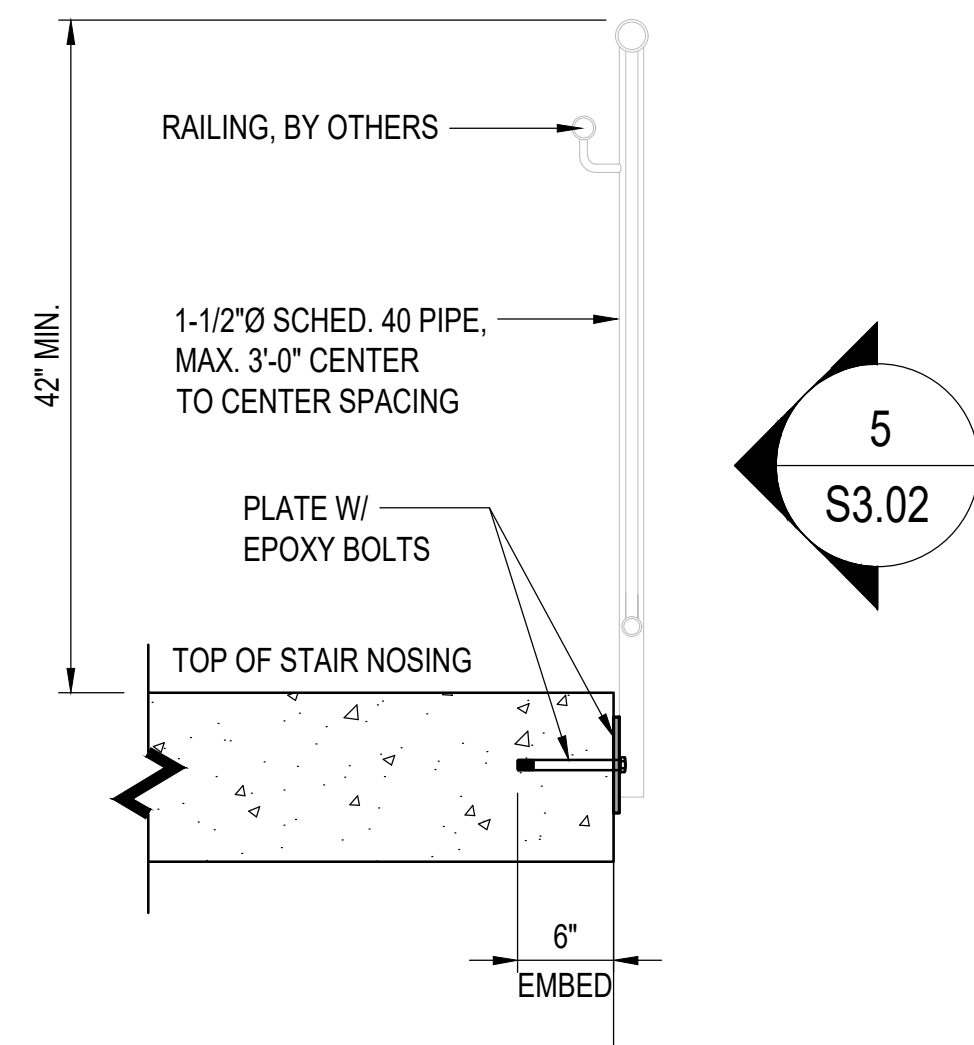


TYP. COLUMN ELEVATION



TYP. LINTEL AT DOOR OPENING DETAIL

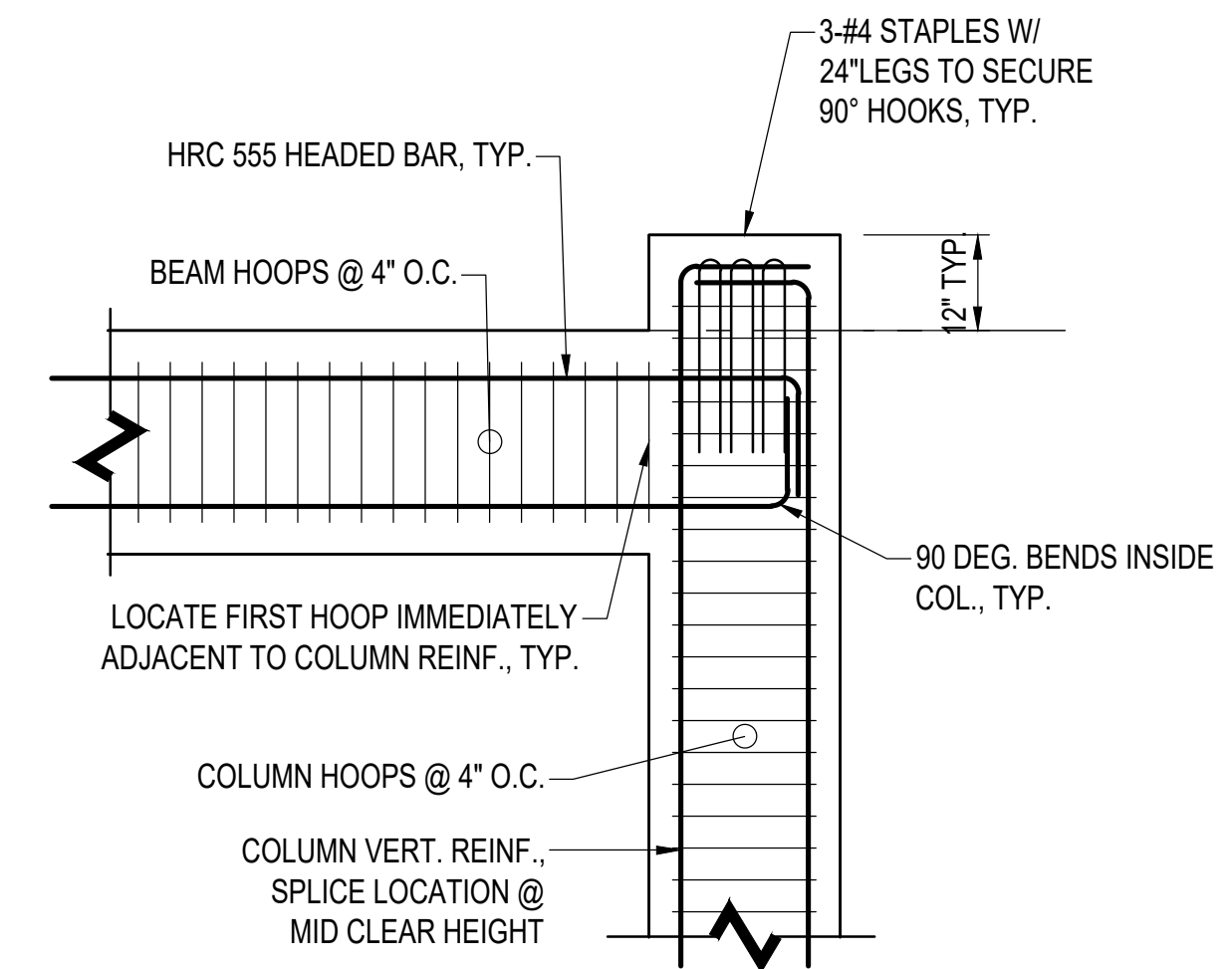
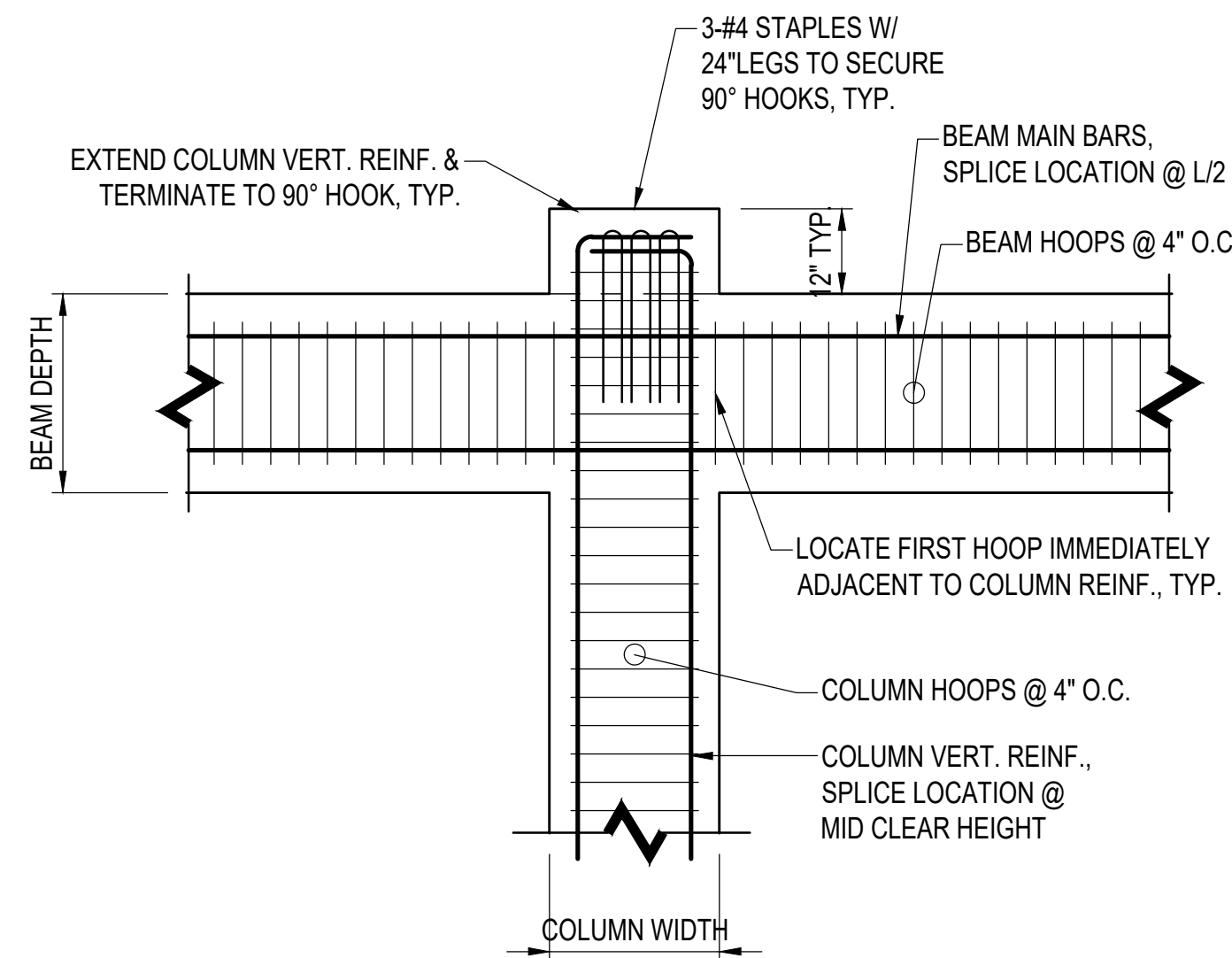
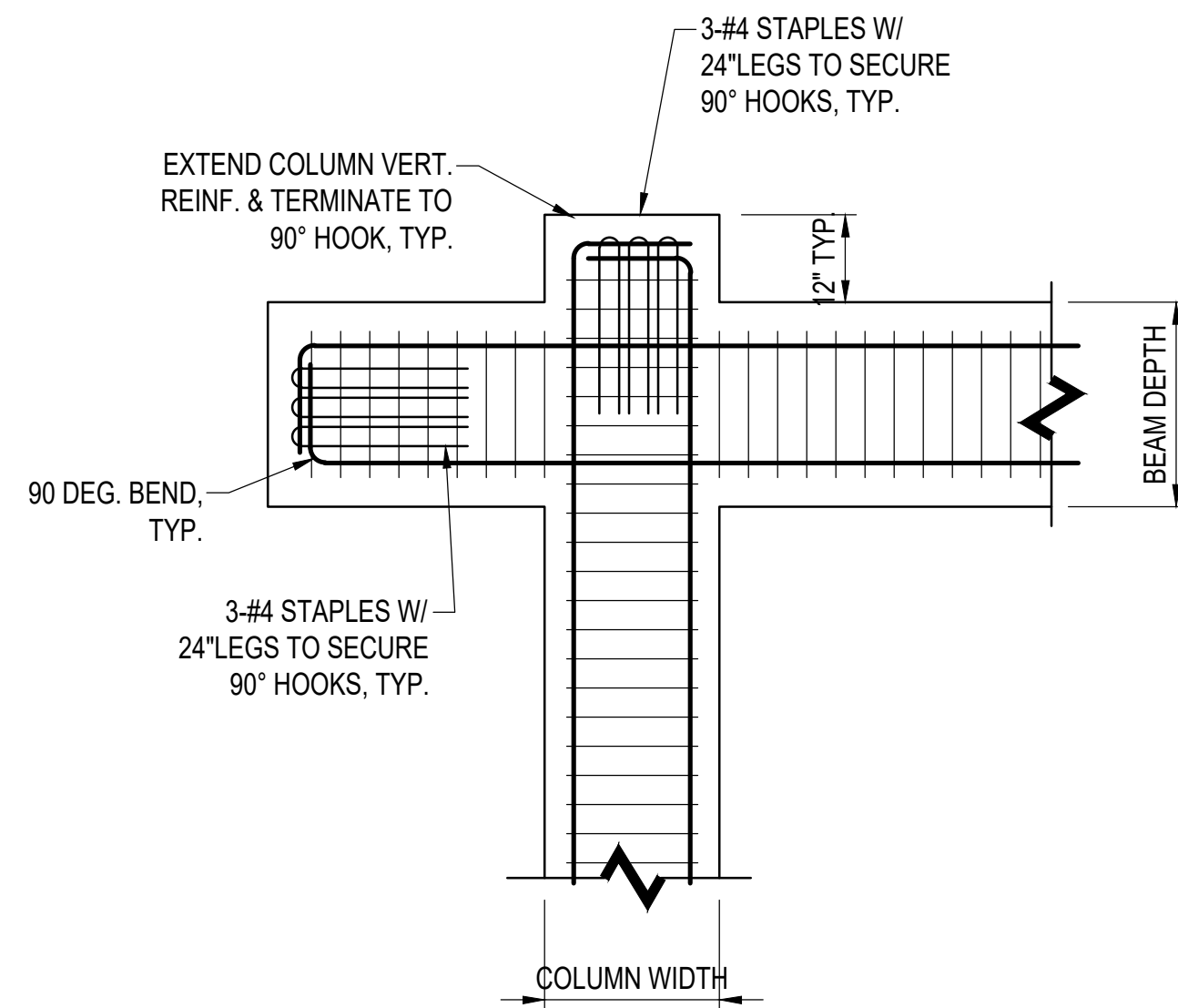
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6 RAILING CONNECTION SECTION DETAIL
S3.02 | S3.02 1" - 1'-0"

5 RAILING CONNECTION ELEVATION DETAIL
S3.02 | S3.02 1" - 1'-0"

4 ROOF CLEARSTORY SECTION
S3.02 | S3.02 1/2" - 1'-0"



3 TYPICAL COLUMN STUB OUT AT BEAM OVERHANG DETAIL
S3.02 | S3.02 1/2" - 1'-0"

2 TYPICAL COLUMN STUB OUT DETAIL
S3.02 | S3.02 1/2" - 1'-0"

1 TYPICAL COLUMN STUB OUT AT EXTERIOR COLUMN
S3.02 | S3.02 1/2" - 1'-0"

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Project:
GMH

Title:
DETAILS

Designed: TG
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 Checked: TG
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S3.02
 Sheet No. ___ of ___

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